Appendix A14.2 Ground Investigation Report





Bus Connects Route 13 Bray to City Centre – Ground Investigation

Client:National Transport Authority (NTA)Client's Representative:JacobsReport No.:20-0399EDate:December 2020Status:Final for Issue

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CONTENTS

Document Control Sheet

Note on: Methods of describing soils and rocks & abbreviations used on exploratory hole logs

1	AUTH	ORITY	4
2	SCOPI	Е	.4
3	DESC	RIPTION OF SITE	.4
4	SITE (4 1	DPERATIONS Summary of site works	5
	4.2	Boreholes	.5
		4.2.1 Light cable percussion boreholes	
		4.2.2 Dynamic sampled boreholes	6
	4.3	Standpipe installations	
	4.4	Slit trenches	.7
	4.5	Surveying	.7
	4.6	Groundwater monitoring	7
5	LABO	RATORY WORK	.7
	5.1	Geotechnical laboratory testing of soils	7
	5.2	Environmental laboratory testing of soils	8
6	GROU	ND CONDITIONS	8
	6.1	General geology of the area	8
	6.2	Ground types encountered during investigation of the site	8
	6.3	Groundwater	
7	REFE	RENCES	0

APPENDICES

Appendix A	Site and exploratory hole location plans
Appendix B	Borehole logs
Appendix C	Slit trench logs and sketches
Appendix D	Slit trench photographs
Appendix E	Geotechnical laboratory test results
Appendix F	Environmental laboratory test results
Appendix G	SPT hammer energy measurement report





Document Control Sheet

Report No.:		20-0399E						
Project Title:		Bus Connects Route 13 Bray to City Centre						
Client:		National Transp	oort Authority (N	ГА)				
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Prepared by:		Reviewed by:		Approved by:				
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The works were conducted in accordance with:

British Standards Institute (2015) BS 5930:2015, Code of practice for site investigations.

BS EN 1997-2: 2007: Eurocode 7 - Geotechnical design - Part 2 Ground investigation and testing.

Geotechnical Society of Ireland (2016), Specification & Related Documents for Ground Investigation in Ireland

Laboratory testing was conducted in accordance with:

British Standards Institute BS 1377:1990 parts 2, 4, 5, 7 and 9



METHODS OF DESCRIBING SOILS AND ROCKS

Soil and rock descriptions are based on the guidance in BS5930:2015, The Code of Practice for Site Investigation.

U	Nominal 100mm diameter undisturbed open tube sample (thick walled sampler).
UT	Nominal 100mm diameter undisturbed open tube sample (thin walled sampler).
Р	Nominal 100mm diameter undisturbed piston sample.
В	Bulk disturbed sample.
LB	Large bulk disturbed sample.
D	Small disturbed sample.
С	Core sub-sample (displayed in the Field Records column on the logs).
L	Liner sample from dynamic sampled borehole.
W	Water sample.
ES / EW	Soil sample for environmental testing / Water sample for environmental testing.
SPT (s)	Standard penetration test using a split spoon sampler (small disturbed sample obtained).
SPT (c)	Standard penetration test using 60 degree solid cone.
(x,x/x,x,x,x)	Blows per increment during the standard penetration test. The initial two values relate to the seating drive (150mm and the remaining four to the 75mm increments of the test length.
(Y for Z/Y for Z)	Incomplete standard penetration test where the full test length was not achieved. The blows 'X' represent the total blows for the given seating or test length 'Z' (mm).
N=X	SPT blow count 'N' given by the summation of the blows 'X' required to drive the full test length (300mm).
HVP / HVR	In situ hand vane test result (HVP) and vane test residual result (HVR). Results presented in kPa.
V VR	Shear vane test (borehole). Shear strength stated in kPa.V: undisturbed vane shear strengthVR: remoulded vane shear strength
Soil consistency description	In cohesive soils, where samples are disturbed and there are no suitable laboratory tests, N values may be used to indicate consistency on borehole logs – a median relationship of Nx5=Cu is used (as set out in Stroud & Butler 1975).
dd-mm-yyyy	Date at the end and start of shifts, shown at the relevant borehole depth. Corresponding casing and water depths shown in the adjacent columns.
\bigtriangledown	Water strike: initial depth of strike.
•	Water strike: depth water rose to.
Abbreviations relatin	g to rock core – reference Clause 36.4.4 of BS 5930: 2015
TCR (%)	Total Core Recovery: Ratio of rock/soil core recovered (both solid and non-intact) to the total length of core run.
SCR (%)	Solid Core Recovery: Ratio of solid core to the total length of core run. Solid core has a full diameter, uninterrupted by natural discontinuities, but not necessarily a full circumference and is measured along the core axis between natural fractures.
RQD (%)	Rock Quality Designation: Ratio of total length of solid core pieces greater than 100mm to the total length of core run
FI	Fracture Index: Number of natural discontinuities per metre over an indicated length of core of similar intensity of fracturing.
NI	Non Intact: Used where the rock material was recovered fragmented, for example as fine to coarse gravel size particles
AZCL	Assessed zone of core loss: The estimated depth range where core was not recovered.
DIF	Drilling induced fracture: A fracture of non-geological origin brought about by the rock coring.
(xxx/xxx/xxx)	Spacing between discontinuities (minimum/average/maximum) measured in millimetres.





Bus Connects Route 13 Bray to City Centre

1 AUTHORITY

On the instructions of Jacobs, ("the Client's Representative"), acting on the behalf of National Transport Authority (NTA) ("the Client"), a ground investigation was undertaken at the above location to provide geotechnical and environmental information to inform the planning stage design and enable the design of Bus Connects Core Bus Corridors.

This report details the work carried out both on site and in the geotechnical and chemical testing laboratories; it contains a description of the site and the works undertaken, the exploratory hole logs and the laboratory test results.

All information given in this report is based upon the ground conditions encountered during the site investigation works, and on the results of the laboratory and field tests performed. However, there may be conditions at the site that have not been taken into account, such as unpredictable soil strata, contaminant concentrations, and water conditions between or below exploratory holes. It should be noted that groundwater levels usually vary due to seasonal and/or other effects and may at times differ to those recorded during the investigation. No responsibility can be taken for conditions not encountered through the scope of work commissioned, for example between exploratory hole points, or beneath the termination depths achieved.

This report was prepared by Causeway Geotech Ltd for the use of the Client and the Client's Representative in response to a particular set of instructions. Any other parties using the information contained in this report do so at their own risk and any duty of care to those parties is excluded.

2 SCOPE

The extent of the investigation, as instructed by the Client's Representative, included boreholes, slit trenches, soil sampling, environmental sampling, groundwater monitoring, in-situ and laboratory testing, and the preparation of a factual report on the findings.

3 DESCRIPTION OF SITE

As shown on the site location plan in Appendix A, the works were conducted along the route of the proposed Bus Connects Route from Bray to the City Centre with investigation points at the N11 junction with the Lower Kilmacud Road, junction of the N11 with the Old Bray Road and within the ground of Ravenswell Primary School in Bray, Co. Wicklow.





Borehole works were undertaken in housing estates off the N11 while slit trenches were undertaken within the central median of the road.

4 SITE OPERATIONS

4.1 Summary of site works

Site operations, which were conducted between 16th October and 2nd November 2020, comprised:

- three light cable percussion boreholes
- one borehole by dynamic (windowless) sampling methods
- a standpipe installation in three boreholes
- six slit trenches

The exploratory holes and in-situ tests were located as instructed by the Client's Representative, as shown on the exploratory hole location plan in Appendix A.

4.2 Boreholes

A total of four boreholes were put down in a minimum diameter of 150mm through soils strata to their completion depths by a combination of methods, including light percussion boring using a Dando Terrier rig and light cable percussion boring by a Dando 2000 rig.

The borehole logs state the methodology and plant used for each location, as well as the appropriate depth ranges.

A summary of the boreholes, subdivided by category in accordance with the methods employed for their completion, is presented in the following sub-sections.

4.2.1 Light cable percussion boreholes

Three boreholes (R13-CP01 – R13-CP03) were put down to completion in minimum 200mm diameter using Dando 2000 light cable percussion boring rigs. All boreholes were terminated either at their scheduled completion depths, or else on encountering virtual refusal on obstructions, including large boulders and weathered bedrock.

Hand dug inspection pits were carried out between ground level and 1.20m depth to ensure boreholes were put down at locations clear of services or subsurface obstructions.





Disturbed (bulk and small bag) samples were taken within the encountered strata. Undisturbed (U100) samples were taken where appropriate and as directed within fine soils. Environmental samples were taken at standard intervals, as directed by the Client's Representative.

Standard penetration tests were carried out in accordance with BS EN 22476-3:2005+A1:2011 at standard depth intervals using the split spoon sampler ($SPT_{(s)}$) or solid cone attachment ($SPT_{(c)}$). The penetrations are stated for those tests for which the full 150mm seating drive or 300mm test drive was not possible. The N-values provided on the borehole logs are uncorrected and no allowance has been made for energy ratio corrections. The SPT hammer energy measurement report is provided in Appendix G.

Any water strikes encountered during boring were recorded along with any changes in their levels as the borehole proceeded.

Where water was added to assist with boring, a note has been added to the log to account for same.

Appendix B presents the borehole logs.

4.2.2 Dynamic sampled boreholes

One borehole (R12-WS01) was put down to completion by light percussion boring techniques using a Dando Terrier dynamic sampling rig. The borehole was put down initially in 150mm diameter, reducing in diameter with depth as required, down to 50mm by use of the smallest sampler.

A hand dug inspection pit was carried out between ground level and 1.20m depth to ensure the boreholes was put down clear of services or subsurface obstructions. The borehole was taken to a depth of 2.0m where it was terminated on encountering virtual refusal.

Standard penetration tests were carried out in accordance with BS EN 22476-3:2005+A1:2011 at standard depth intervals using the split spoon sampler ($SPT_{(s)}$) or solid cone attachment ($SPT_{(c)}$). The penetrations are stated for those tests for which the full 150mm seating drive or 300mm test drive was not possible. The *N*-values provided on the borehole logs are uncorrected and no allowance has been made for energy ratio corrections. The SPT hammer energy measurement report is provided in Appendix G.

Groundwater was not encountered during boring.

Appendix B presents the borehole logs.

4.3 Standpipe installations

A groundwater monitoring standpipe was installed in BH01-BH03.





Details of the installations, including the depth range of the response zone, are provided in Appendix B on the individual borehole logs.

4.4 Slit trenches

Six slit trenches (R13-SLT01-R13-SLT03, R13-SLT03A and R13-SLT04-R13-SLT15) were excavated by a combination of hand digging and mechanical excavation using a compact 3t tracked excavator fitted with a 600mm wide toothless bucket, to locate and identify buried services at the site.

Drawing of the trenches and the locations of services encountered during excavation are shown along with the slit trench logs in Appendix D, with photographs presented in Appendix E.

4.5 Surveying

The as-built exploratory hole positions were surveyed following completion of site operations by a Site Engineer from Causeway Geotech. Surveying was carried out using a Trimble R6 GPS system employing VRS and real time kinetic (RTK) techniques.

The plan coordinates (Irish Transverse Mercator) and ground elevation (mOD Malin (Irl)) at each location are recorded on the individual exploratory hole logs. The exploratory hole plan presented in Appendix A shows these as-built positions.

4.6 Groundwater monitoring

Following completion of site works, groundwater monitoring was conducted on one round. Ground water monitoring was carried out using a water interface probe.

The monitoring records are presented in Section 6.3.

5 LABORATORY WORK

Upon their receipt in the laboratory, all disturbed samples were carefully examined and accurately described and their descriptions incorporated into the borehole logs.

5.1 Geotechnical laboratory testing of soils

Laboratory testing of soils comprised:

• **soil classification:** moisture content measurement, Atterberg Limit tests and particle size distribution analysis.





• soil chemistry: BRE Test Suite B

Laboratory testing of soils samples was carried out in accordance with British Standards Institute: *BS 1377, Methods of test for soils for civil engineering purposes; Part 1 (2016), and Parts 2-9 (1990).*

The test results are presented in Appendix H.

5.2 Environmental laboratory testing of soils

Environmental testing, as specified by the Client's Representative was conducted on selected environmental soil samples by Chemtest at its laboratory in Newmarket, Suffolk.

Testing was carried out on a number of samples according to Engineer's Ireland Suite E and Suite H including testing for a range of determinants:

- Metals
- Speciated total petroleum hydrocarbons (TPH)
- Speciated polycyclic aromatic hydrocarbons (PAH)
- Cyanides
- Asbestos screen
- pH
- Waste acceptance criteria (WAC) testing

Results of environmental laboratory testing are presented in Appendix F.

6 **GROUND CONDITIONS**

6.1 General geology of the area

Published geological mapping indicate the superficial deposits underlying the site comprise Glacial Till, fluvioglacial sands and gravels and made ground. These deposits are underlain by Leinster Granite in the north of the route and by the Maulin Formation in the section of the route.

6.2 Ground types encountered during investigation of the site

A summary of the ground types encountered in the exploratory holes is listed below, in approximate stratigraphic order:

• **Paved surface:** boreholes R13-CP01 and R13-CP02 encountered 100-200mm of macadam surfacing.





- **Topsoil:** encountered in R13-CP03, R13-WS01 and R13-SLT01-R13-SLT05 ranging in thickness from 100-500mm.
- **Made Ground (sub-base):** approximately 200 to 1200mm of aggregate fill beneath the paved surface in R13-CP01 and R13-CP02,
- Made Ground (fill): reworked sandy gravelly clay or silty sandy gravel or brown fine to coarse sand fill encountered at all locations to a maximum depth of 3.50m in R13-CP03. Varying amounts of red brick, concrete, tin and plastic fragments were encountered in R13-CP03, R13-SLT01, R13-SLT02, R13-SLT03 and R13-SLT03A to a maximum depth of 2.30m in R13-CP03.
- **Fluvioglacial deposits:** typically, medium dense sands and gravels interspersed with layers of sandy gravelly clay encountered at all borehole locations.
- **Glacial Till:** sandy gravelly clay or silt, frequently with low cobble content, typically firm or stiff in upper horizons, becoming very stiff with increasing depth.

6.3 Groundwater

Details of the individual groundwater strikes, along with any relative changes in levels as works proceeded, are presented on the exploratory hole logs for each location.

Groundwater was encountered during percussion boring in R13-CP02 through soil as a water strike at 2.30m in R13-CP02.

Groundwater was not noted during drilling at any of the other borehole locations. However, it should be noted that the casing used in supporting the borehole walls during drilling may have sealed out any additional groundwater strikes and the possibility of encountering groundwater at other depths during excavation works should not be ruled out.

Groundwater was not noted during excavation of any of the slit trenches.

Subsequent groundwater monitoring of the standpipe installations recorded water levels as shown in Table 1.

Date	Water level (mbgl)									
Date	R13-CP01	R13-CP02	R13-CP03							
19/11/2020	2.47	2.24	Dry							

 Table 1: Groundwater monitoring

Seasonal variation in groundwater levels should also be factored into design considerations and continued monitoring of the installed standpipes will give an indication of the seasonal variation.





7 **REFERENCES**

Geotechnical Society of Ireland (2016), Specification & Related Documents for Ground Investigation in Ireland

IS EN 1997-2: 2007: Eurocode 7 - Geotechnical design - Part 2 Ground investigation and testing. National Standards Authority of Ireland.

BS 5930: 2015: Code of practice for ground investigations. British Standards Institution.

BS EN ISO 14688-1:2018: Geotechnical investigation and testing. Identification and classification of soil. Part 1 Identification and description.

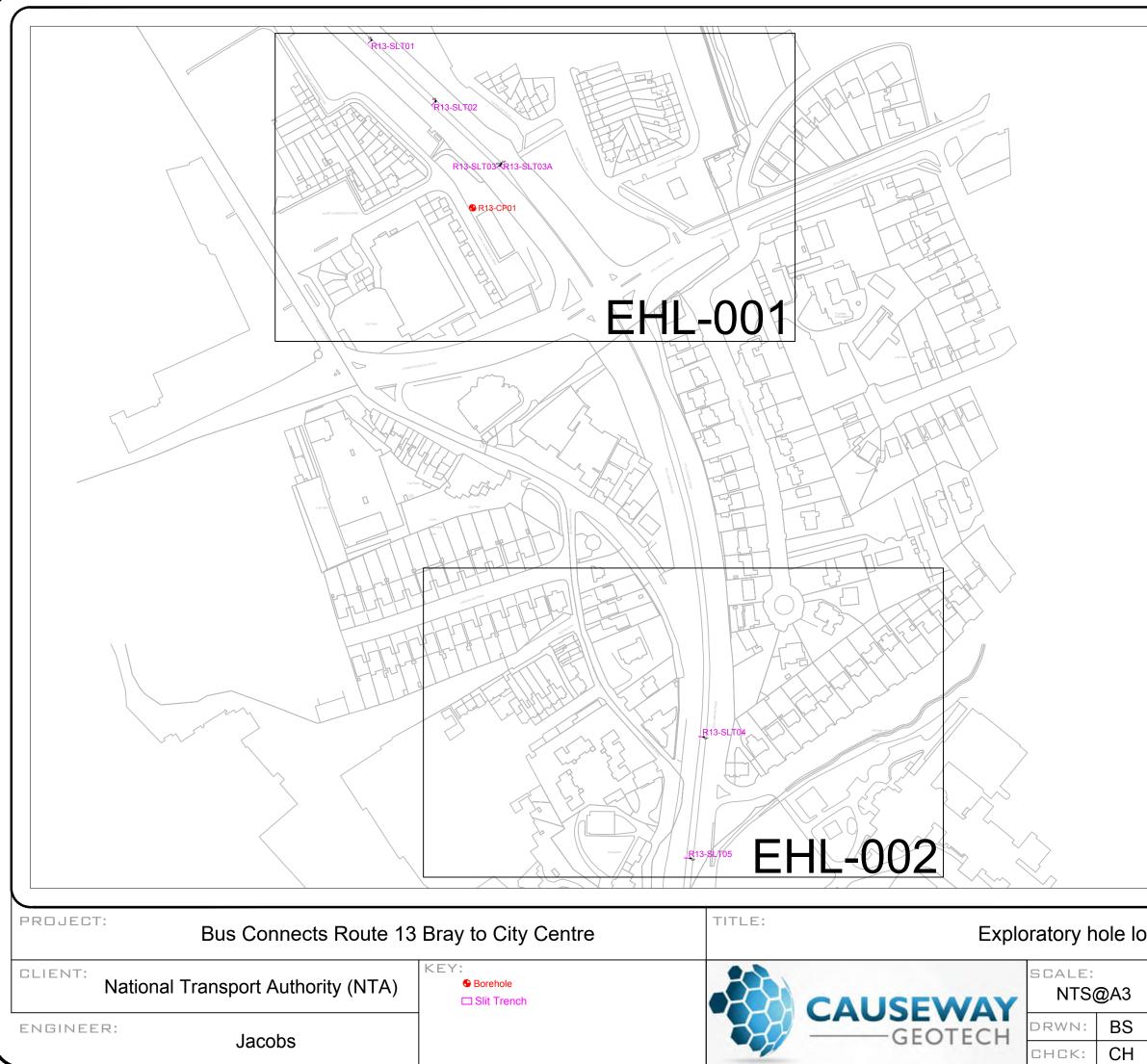
BS EN ISO 14688-2:2018: Geotechnical investigation and testing. Identification and classification of soil. Part 2 Principles for a classification.

BS 1377: 1990: Methods of test for soils for civil engineering purposes. British Standards Institution.

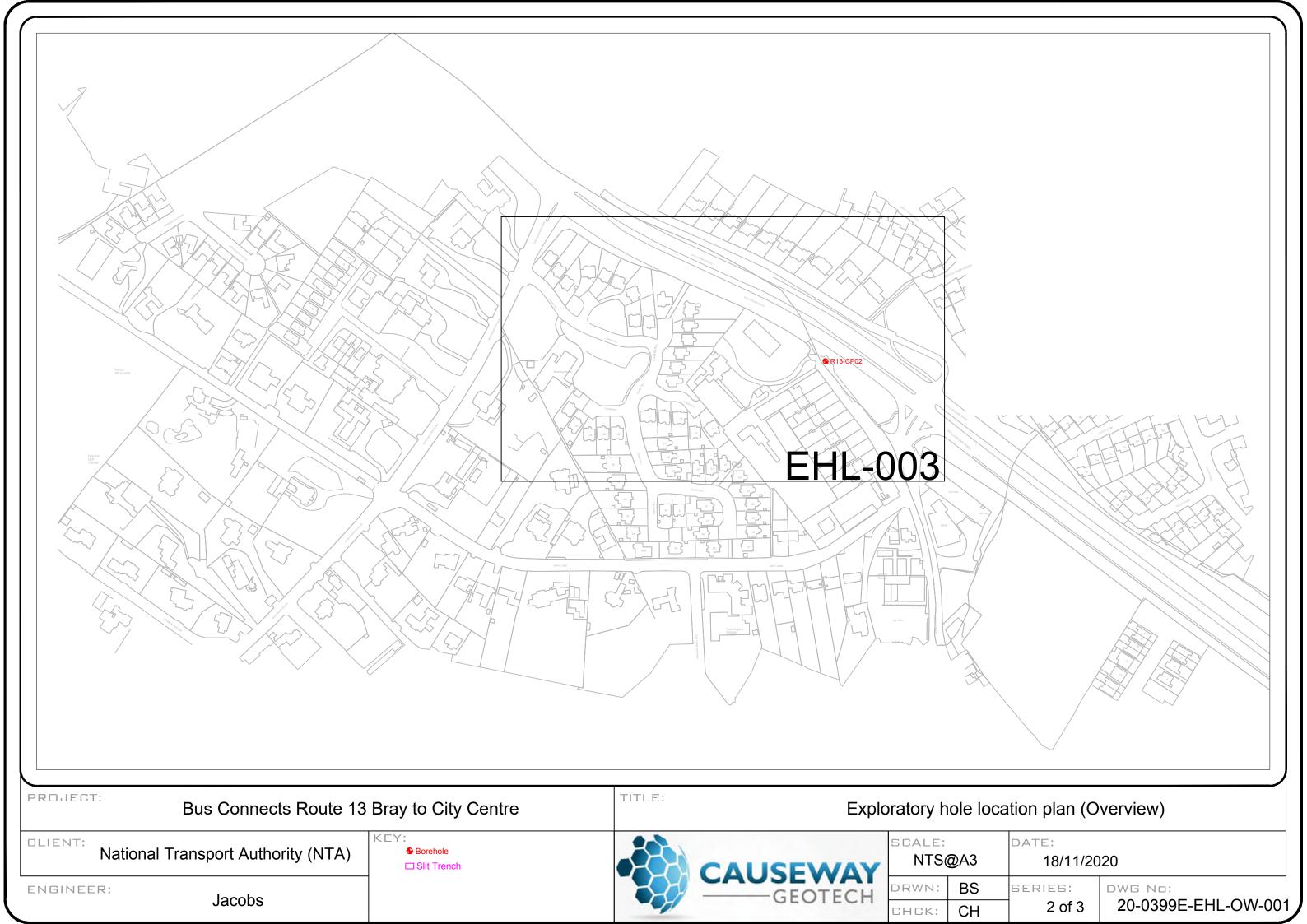
BS EN ISO 14689-1:2018: Geotechnical investigation and testing. Identification and classification of rock. Identification and description.

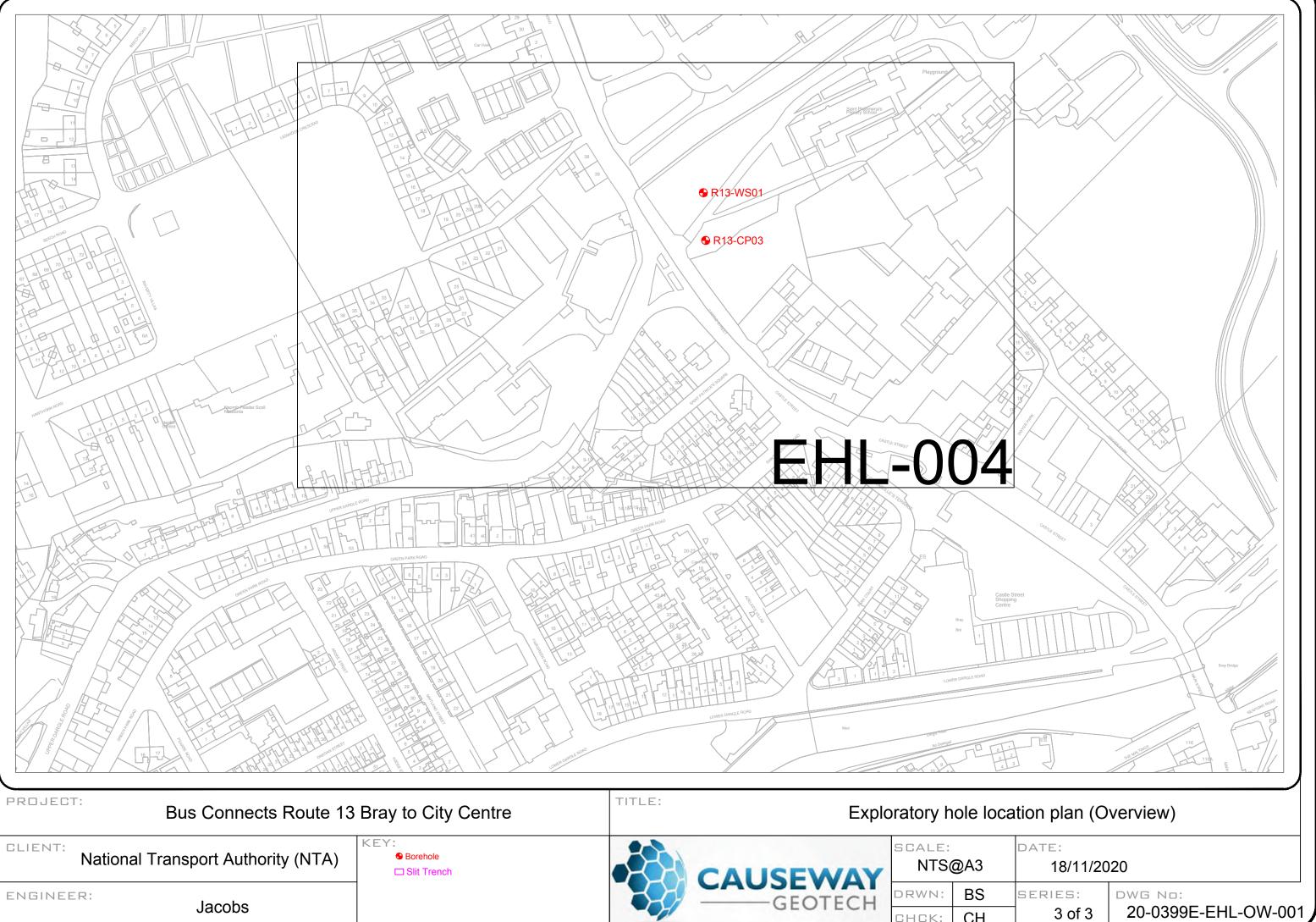
BS EN ISO 22476-3:2005+A1:2011: Geotechnical investigation and testing. Field testing. Standard penetration test.

APPENDIX A SITE AND EXPLORATORY HOLE LOCATION PLAN

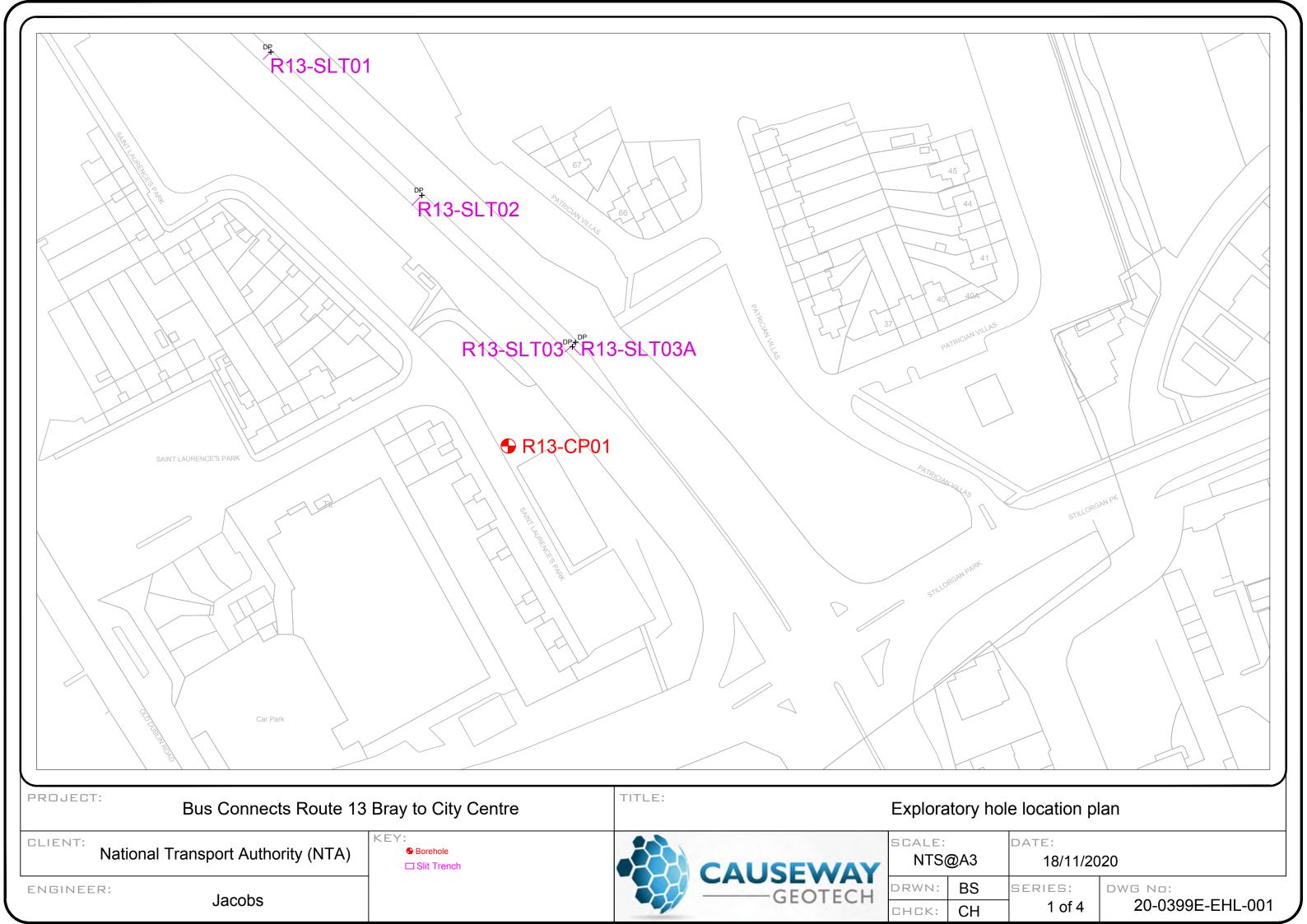


	tion plan (O	varviouv)
oca	tion plan (O	
	DATE: 18/11/20	20
	series: 1 of 4	DWG NO: 20-0399E-EHL-OW-001





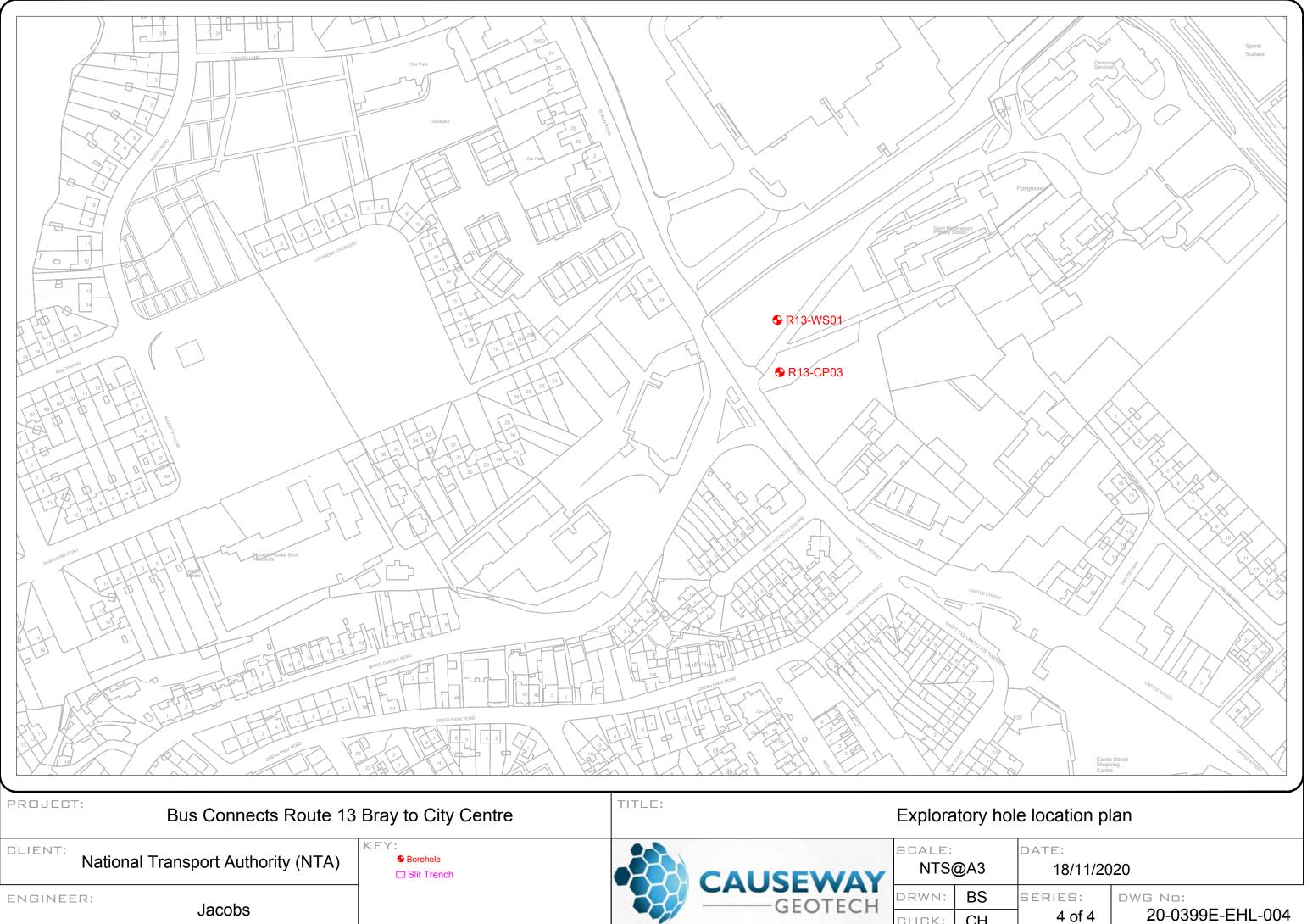
ROJECT:	Bus Connects Route 13	Bray to City Centre	TITLE:	Explo	oratory h	ole lo
LIENT: Nation	nal Transport Authority (NTA)	KEY: Borehole Slit Trench			SCALE: NTS(
NGINEER:	laasha			CAUSEWAY GEOTECH	DRWN:	BS
	Jacobs			GLOTLEIT	CHCK:	СН





PROJECT: Bus Connects Route 13	Bray to City Centre	TITLE:	Exploratory
CLIENT: National Transport Authority (NTA)	KEY: ● Borehole □ Slit Trench		SEWAY SCALE: NTS@A3
ENGINEER: Jacobs		CAU	GEOTECH DRWN: BS GHGK: CH







APPENDIX B BOREHOLE LOGS

		GEOT	ECH	1		20-0399E Client: National Transport Authorit Client's Rep: Jacobs										D-0399E Client: National Transport Authority (NTA) Client's Rep: Jacobs			Boreho R13-C	
Meth Cable Per		Plant Used Dando 2000	Top (m)	Base 4.8		Coord	linates	Final De	pth: 4.80 m	Start Date: 28/10	0/2020	Driller: BM	Sheet 1							
		Danue 2000	0.00	4.0			.6.36 E 9.44 N	Elevatio	n: 48.99 mOD	End Date: 28/10	0/2020	Logger: GH	Scale: FIN							
Depth (m)	Sample / Tests	Field Record	s	Depth D	Vater Depth (m)	Level mOD	Depth (m)	Legend		Description			Backf	ill						
						48.89	0.10		BITMAC MADE GROUND: Gr	ey sandy angular fine	e to coars	e GRAVEL of	-	000						
).50).50	B1 ES7					48.69	- 0.30		limestone. Sand is fi MADE GROUND: So		dy gravell [,]	y CLAY. Sand is fine		0.5						
1.00 1.00 1.20 1.20 - 1.65	B2 ES8 D11 SPT (S)	N=9 (2,3/2,2,2,3) Han 0643	nmer SN =	0.00 [47.79	- - - - -			brown slightly sandy bangular to subround				1.0						
2.00 2.00 2.00 2.00 - 2.45	B3 D9 ES12 SPT (S)	N=21 (4,4/7,5,4,5) Ha 0643	mmer SN =	1.50 [Dry		-							2.0						
3.00 3.00 3.00 3.00 - 3.45	B4 D13 ES10 U15	Ublow=25 90%		3.00 [Dry		-							3.0 3.5						
.00 .00 - 4.20	B5 SPT (S)	N=50 (7,25/50 for 50r Hammer SN = 0643	nm)	3.00 [Dry	44.89	- - - 4.10		Sand is fine to coars	ly sandy gravelly CLA e. Gravel is subangul ologies. Cobbles are	lar to subi	rounded fine to		• 4.(
4.50 4.50 4.70 - 4.78	B6 D14 SPT (S)	N=50 (25 for 35mm/5 50mm) Hammer SN =		3.00 I	Dry	44.39 44.19	- 4.60 - 4.80		lithologies. Dense grey sandy G (Possible bedrock)	RAVEL of mixed litho	-	nd is fine to coarse		4.5						
			0043				- - - -			End of Borehole a	t 4.80m			5.						
							-							6.0						
							-							6.5						
							-							7.						
	Wate	r Strikes		Chise	lling	Details		Remarks												
ruck at (m)) Time (min) Rose to	(m) From (To (m		e (hh:mm)	Hand dug ir	nspection pit excavate water encountered.	ed to 1.20m.										
Casing To (m)	Details Diameter	Water Added From (m) To (m																		
3.00	200		1					Temple				1 a a b 1 1 - d - 4 - 4								
								Ierminati	on Reason			Last Updated		\sim						

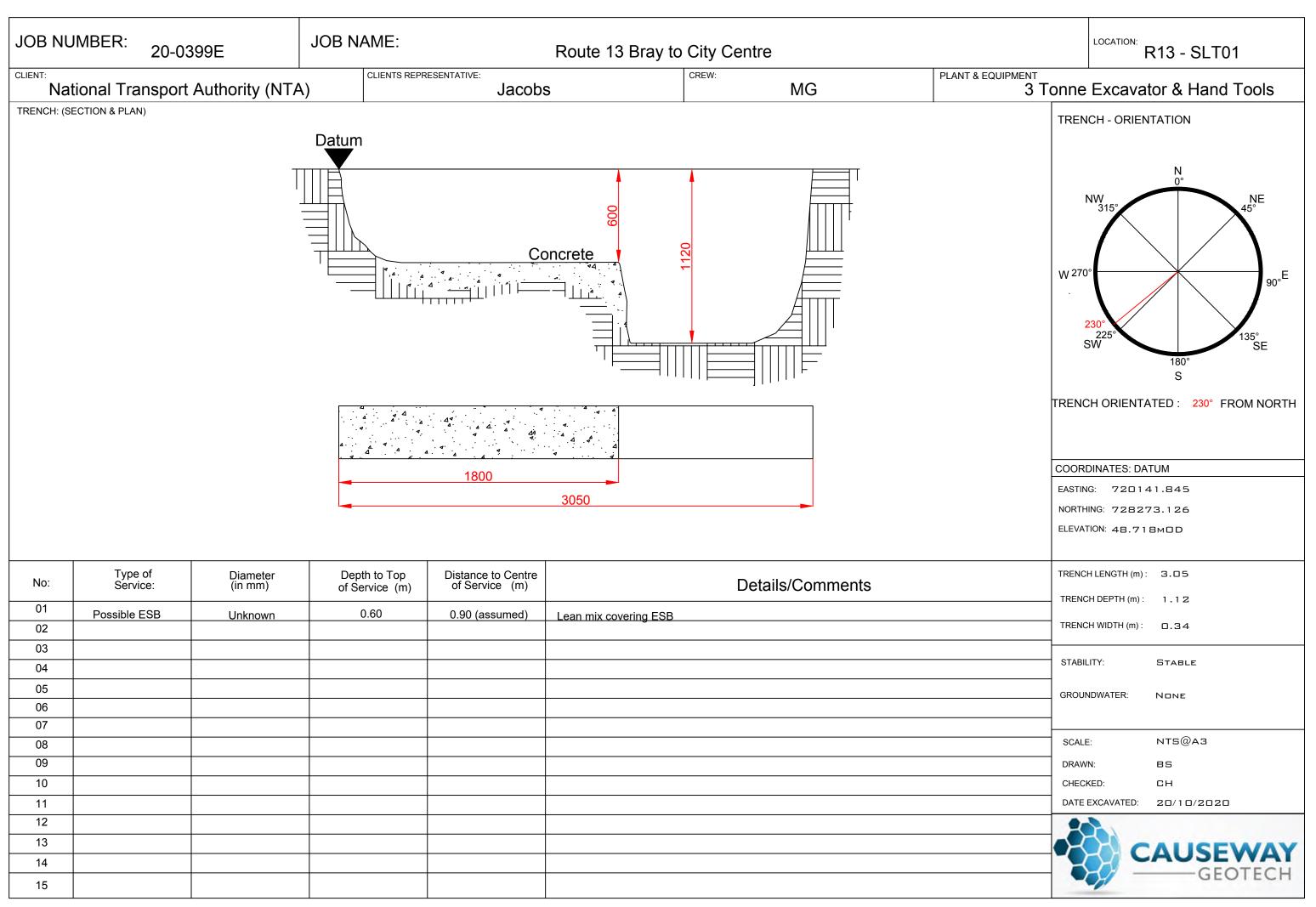
		GEOT	ECH		:	20-0	ct No. 399E	Project Name: Bus Connects Route 13 Bray to City Centre Client: National Transport Authority (NTA) Client's Rep: Jacobs						OE Client: National Transport Authority (NTA) Client's Rep: Jacobs			R	orehole	02
Metho Cable Perci		Plant Used Dando 2000	Top (m) 0.00	Base 2.7		Coord	inates	Final De	pth: 2.70 m	Start Date:	27/10/2020	Driller: BM		Sheet 1 o					
Cable Ferci	0331011	Dando 2000	0.00	2.7	-		0.70 E 5.61 N	Elevatio	n: 55.91 mOD	End Date:	27/10/2020	Logger: GH		Scale: 1: FINAL					
Depth (m)	Sample / Tests	Field Record	ds	Depth I		Level mOD	Depth (m)	Legend		Des	cription		Water	Backfill					
					5	5.70	- 0.20		BITMAC										
0.50 0.50	B1 ES5				5	5.70	-		MADE GROUND: Gr mixed lithologies. Sa			ılar GRAVEL of			0.5 ·				
1.00 1.00	B2 ES6					4 70	- 1.20								1.0 -				
1.20 1.20 - 1.65		N=10 (2,3/2,2,3,3) Ha 0643	ammer SN =	0.00		4.70	- 1.20	0 × 0 × 0 × 0 0 × 0 × 0 × 0 0 × 0 × 0 × 0 × 0 0 × 0 × 0 × 0 × 0 0 × 0 × 0 × 0 × 0	Medium dense brow mixed lithologies wi Cobbles are subrour	th low cobble	content. Sand i				• 1.5 •				
2.00	B3				5	3.90	- 2.00	a X: • a X 0	Medium dense brow	vn sandy silty	subangular to s	ubrounded GRAVE	L		2.0 -				
2.00 2.00	D10 ES7						-	a 9 0 0	of mixed lithologies Cobbles are subrour	with low cob	ble content. San								
2.00 - 2.45		N=10 (2,3/2,3,3,2) Ha 0643	ammer SN =	1.50		3.40	- 2.50	ê 0 0			-	1. 6.			2.5 .				
2.50	В4	Strike at 2.30m				3.20	- 2.70		Dense grey sandy G (Possible bedrock)		-	and is fine to coarse	2.		-				
2.50 3.00 - 3.18	ES8	N=50 (25 for 50mm/					-			End of Bore	hole at 2.70m								
3.00 - 3.18	5FT (5)	125mm) Hammer SN		1.50 1			-								3.0 -				
															3.5 ·				
							-								4.0 -				
							-								4.5 ·				
							-												
							-								5.0 -				
							-								5.5 ·				
							50 50												
															6.0 -				
							-								6.5 ·				
							-								7.0 -				
	Michi	r Strikes	1		lling D	ot-il-	r	Pomeri											
Struck at (m) Ca 2.30) Time (min) Rose to 20 2.20		(m)	Iling D To (m) 2.70	Time		Remarks Hand dug ir	nspection pit excavate	ed to 1.20m.									
2.30			2.5	-	2.70		.1.00												
Casing De	etails	Water Added																	
To (m) [1.50	Diameter 200	From (m) To (m	<u>ı)</u>					Faure 1: 11					-						
								Terminated	on Reason			Last Updated		AC	`C				

	- 1	GEOT	ECH			20-0	ct No. 399E	Project Name: Bus Connects Route 13 Bray to City Centre Client: National Transport Authority (NTA) Client's Rep: Jacobs	Borehole ID R13-CP03
Method Cable Percus		Plant Used Dando 2000	Top (m) 0.00	Base 7.0		72600	2.77 E	Final Depth: 7.00 m Start Date: 16/10/2020 Driller: BM Elevation: 10.56 mOD End Date: 17/10/2020 Logger:	Sheet 1 of 1 Scale: 1:40 FINAL
Depth (m)	Sample / Tests	Field Records		Casing Depth	Water Depth	Level mOD	Depth (m)	Legend Description	ਲੇ ਲੇ Backfill ≥
0.50 6	31 259			(m)	(m)	10.46	0.10	TOPSOIL MADE GROUND: Brown very sandy silty subangular to subrounded fine to coarse GRAVEL of mixed lithologies with fragments of red brick. Sand is fine to coarse.	0.5 -
1.00 F 1.00 F 1.20 F	32 55 5510 515 5PT (S)	N=2 (0,1/0,1,0,1) Ham 0643	mer SN =	1.00	Dry	9.36	- - - - - - -	MADE GROUND: Very soft greyish brown sandy gravelly CLAY with frequent fragments of brick. Sand is fine to coarse. Gravel is subangular fine to coarse of mixed lithologies.	1.0
2.00 I 2.00 I	33 D16 ES11 GPT (S)	N=4 (0,1/1,1,1,1) Ham 0643	mer SN =	1.50	Dry	8.26	- - - - 2.30 -	MADE GROUND: Soft brown sandy gravelly CLAY. Sand is fine to coarse. Gravel is subangular fine to coarse of mixed lithologies.	2.0 -
3.00	34 5512 J20	Ublow=20 100%		1.50	Dry	7.06	- - - - - - - - - - - - - - - - - - -	Very stiff brown slightly sandy slightly gravelly SILT. Sand is fine to coarse. Gravel is subrounded fine to medium of mixed lithologies.	3.0 -
4.00 I 4.00 I		N=30 (4,5/7,7,7,9) Har 0643	nmer SN =	: 3.00	Dry		- - - - - - - - -		* 4.0 * 4.0 * 5.0 - 5.0 * 5.0 - 5.0
5.00 I 5.00 E	36 D18 ES14 SPT (S)	N=38 (6,6/8,9,9,12) Ha = 0643	ammer SN	3.00	Dry	5.06	- - - - - - 5.50	Dense brown sandy silty subrounded fine to coarse GRAVEL of mixed lithologies. Sand is fine to coarse.	5.0 -
	37 J21	Ublow=50 60%		3.00	Dry	4.06	- - - - - - - 6.50	Dense brown gravelly clayey fine to coarse SAND. Gravel is	6.0
7.00	38 019 SPT (S)	N=50 (9,11/50 for 225 Hammer SN = 0643	mm)	3.00	Dry	3.56	- - - 7.00 - -	subrounded fine to coarse of mixed lithologies. End of Borehole at 7.00m	7.0 -
Struck at (m) Casi	ng to (m	Strikes Time (min) Rose to (Water Added From (m) To (m)			elling To (g Details m) Tim		Remarks Hand dug inspection pit excavated to 1.20m.	
	aneter							Termination Reason Last Updated Terminated on refusal. 17/12/2020	AGS

•	/ -	GEOT	ECH			20-0	ct No. 399E	Project Name: Bu Client: Na Client's Rep: Jac	tional Transpo	ute 13 Bray to C rt Authority (NT		R	orehole ID 13-WS01
Metho Light Percu		Plant Used Dando Terrier	Top (m) 0.00	Base 2.0			linates	inal Depth: 2.	00 m Start Da	te: 19/10/2020	Driller: JC		heet 1 of 1 Scale: 1:50
							01.49 E 53.98 N	levation: 13.64	mOD End Dat	e: 19/10/2020	Logger:		FINAL
Depth (m)	Sample / Tests	Field Record	5	Casing Depth (m)	Water Depth (m)	Level mOD	Depth (m)	Legend		Description		Water	Backfill
0.50 0.50 - 1.30 1.00	ES1 B2 ES3					13.14	- 0.50			elly SILT. Sand is find e to coarse of mixed	e to coarse. Gravel is d lithologies.	5	0.5
1.20 1.20 - 1.65 1.30 - 1.60 1.60 - 2.00	D4 SPT (C) B5 B6	N=13 (3,4/3,3,3,4) Ha 0696	mmer SN =	0.00	Dry	12.34	1.30	XXX XXX Medium den		ry sandy silty subar AVEL of mixed litho			1.5
2.00 2.00 - 2.44	ES7 SPT (C)	N=50 (8,10/50 for 285 Hammer SN = 0696	imm)	0.00	Dry	11.64	- 2.00		End of I	Borehole at 2.00m		_	2.0
							- - - - -						2.5
							- - - - -						3.5
							- - - - -						4.0
							-						4.5
							- - - -						5.5
							- - - - -						6.0
							- - - -						6.5
													7.0
							- - - 						8.0
							- - - -						8.5
							-						9.0
itruck at (m) Ca		r Strikes J) Time (min) Rose to (n)	Detail Diam	eter _{Hai}		ction pit excavated to r encountered.	1.20m.				1
							r minatio minated o				Last Updated 17/12/2020		AGS

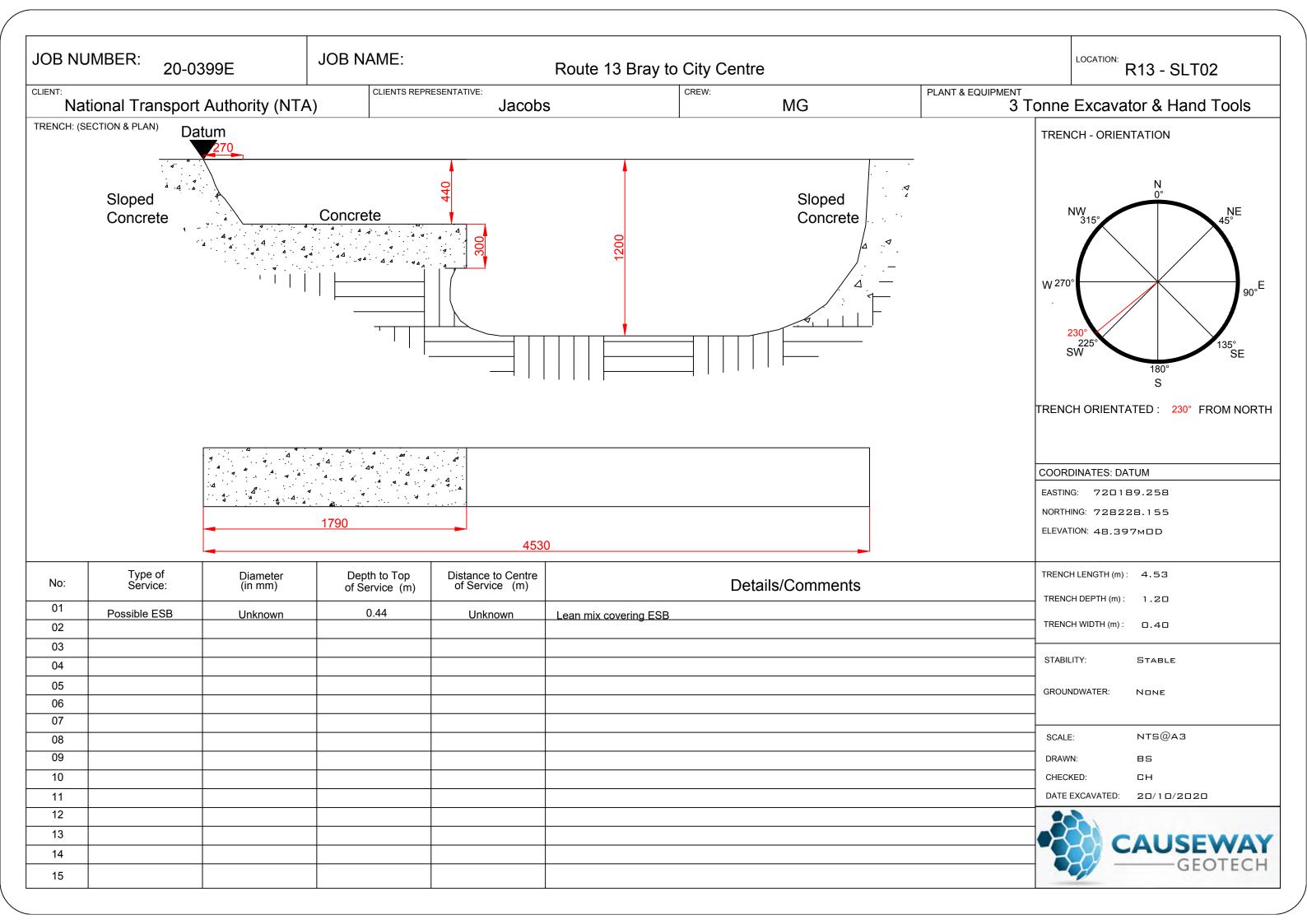
APPENDIX C SLIT TRENCH LOGS AND DRAWINGS

	CAUS	EWAY EOTECH	20-0	ect No. D399E dinates	Bus Co Client:	: Name: nnects Route 13 Bray to City Centre al Transport Authority (NTA)			rial Pit ID 13-SLT01
Method: Slit Trenching			7282	41.85 E 73.13 N	Client's	Sheet 1 of 2 Scale: 1:25			
Plant: 3T Tracked Excavator				vation 2 mOD	Date: 20/10/	2020	Logger: MG		FINAL
Depth	Sample /	Field Records	Level	Depth	Legend	Description		Water	
<u>(m)</u>	Tests		(mOD) 48.52	(m) 0.20		TOPSOIL MADE GROUND: Soft to firm light brown and brow with low cobble content and fragments of brick and fine to coarse. Gravel is subangular to subrounded mixed lithologies. Cobbles are subrounded of mixe	d concrete. Sand is fine to coarse of		
0.50 0.50	B3 ES1		48.12	0.60		MADE GROUND: Soft to firm dark brown and greyi: gravelly CLAY with low cobble content. Sand is fine subangular to subrounded fine to coarse of mixed l are subrounded of mixed lithologies.	sh brown sandy to coarse. Gravel is		0.5
1.00 1.00	B4 ES2		47.60	 1.12		End of trial pit at 1.12m			1.0
				-					
				- - - - - -					
				- - - - -					-
				- - - - -					2.5 —
				- - - - -					
				- - - - -					-
				-					3.5 —
				- - - - - -					- - 4.0
				-					-
				-					4.5
				-					-
Wate Struck at (m)	r Strikes Remarks	Depth: 1.12 Width: 0.34 Length: 3.05		oundwate		tered. t 0.60m on western side of trench.			1
		Stability: Stable		ination Re		uction of the engineer.	Last Updated		AGS

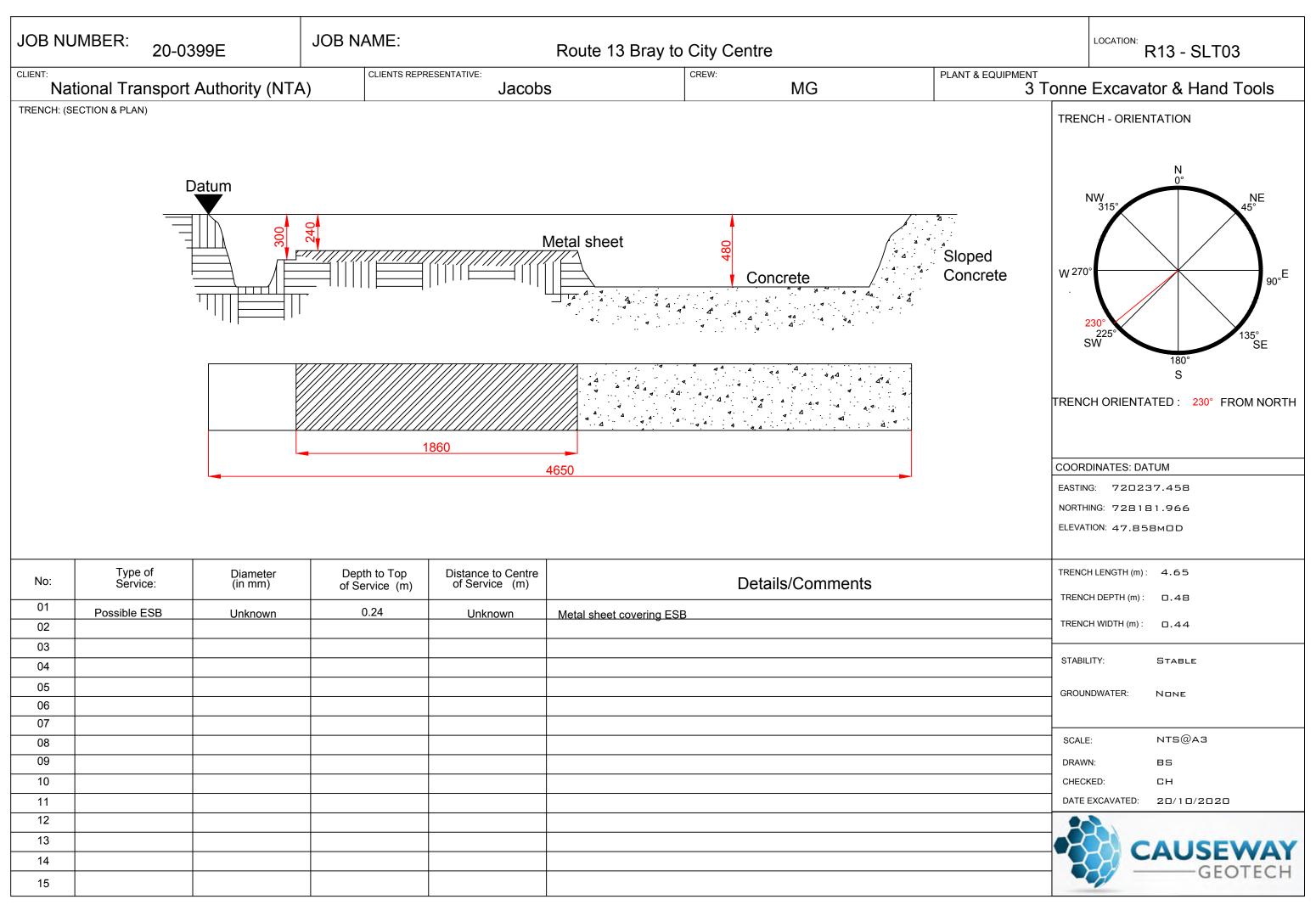


No:	Type of Service:	Diameter (in mm)	Depth to Top of Service (m)	Distance to Centre of Service (m)	Details/Comments
01	Possible ESB	Unknown	0.60	0.90 (assumed)	Lean mix covering ESB
02					
03					
04					
05					
06					
07					
08					
09					
10					
11					
12					
13					
14					
15					

	CALIS			ect No. 0399E	-	: Name: nnects Route 13 Bray to City Centre			Frial Pit ID		
	G	EWAY Eotech	Coordinates		Client: National Transport Authority (NTA)				R13-SLT02		
Method:			72018	39.26 E		s Representative:		-			
Slit Trenching			72822	28.16 N	Jacobs				heet 1 of 1 Scale: 1:25		
Plant:			Flev	ation	Date:	-	scale: 1:25				
3T Tracked Ex	cavator		Elevation 48.40 mOD		20/10/	2020	Logger: MG		FINAL		
Depth	Sample /	Field Records	Level Depth					ater			
		Field Records			Legend	Description TOPSOIL MADE GROUND: Soft slightly sandy gravelly silty CLL coble content and fragments of brick, concrete, tir fine to coarse. Gravel is subangular to subrounded f limestone of mixed lithologies. Cobbles are subrour lithologies. MADE GROUND: Firm to stiff brown and dark greyis sandy gravelly CLAY with medium cobble content. S Gravel is subangular to subrounded fine to coarse o Cobbles are subangular of mixed lithologies.	AY with medium h, and plastic. Sand is fine to coarse of ided of mixed h brown slightly and is fine to coarse.				
				- - - - - - - - - - -					3.5		
				- - - - - -							
				-							
Wate Struck at (m)	er Strikes Remarks	Depth: 1.20 Width: 0.40 Length: 4.53	Lean	oundwate mix concr	ete enco	tered. untered at 0.35mbgl on eastern side of trench.					
		Stability: Stable		ination Re		uction of the engineer.	Last Updated		AGS		

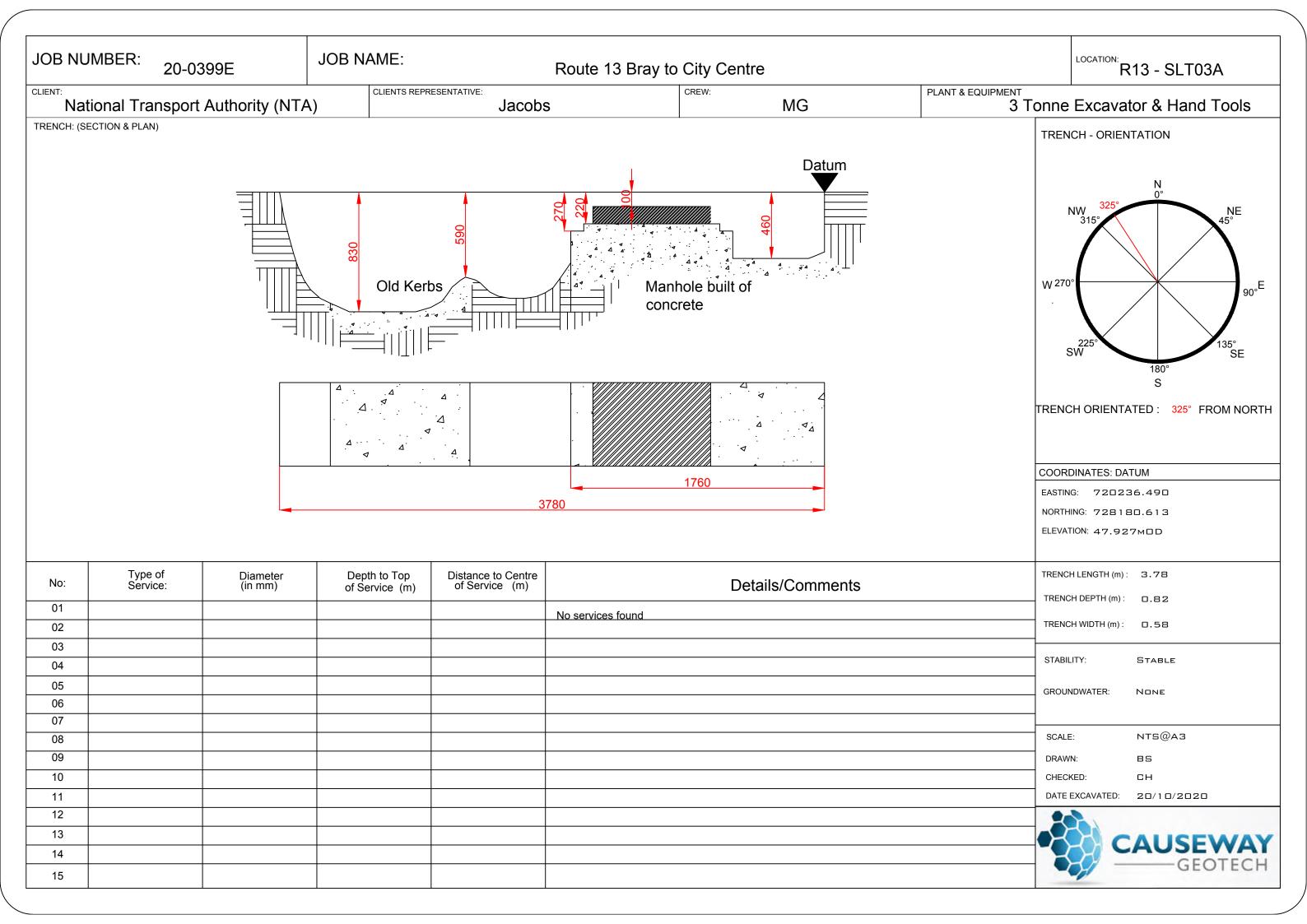


CAUSEWAY GEOTECH Method: Slit Trenching			20-0399E Coordinates		Project Name: Bus Connects Route 13 Bray to City Centre Client: National Transport Authority (NTA) Client's Representative: Jacobs				Trial Pit ID 13-SLT03 heet 1 of 1 Scale: 1:25	
Plant:				vation	Date:	2020	Logger:		FINAL	
3T Tracked Exca Depth		5110 I	47.86 mOD		20/10/		MG	ter		
0.40	Sample / Tests B2 ES ES1 S1 Strikes	Field Records	Level (mOD) 47.66 47.51 47.38	Depth (m)	Legend	Description TOPSOIL MADE GROUND: Soft slightly sandy gravelly silty CL cobble content and fragments of brick, concrete, ti fine to coarse. Gravel is angular to subangular fine- limestone. Cobbles are subangular of limestone. CONCRETE End of trial pit at 0.48m	n, and plastic. Sand is	Mater		
Struck at (m)	Remarks	 Depth: 0.48 Width: 0.44 		roundwate	er encoun	tered.				
		Length: 4.65 Stability: Stable		ination Re		obstruction	Last Updated 17/12/2020		AGS	

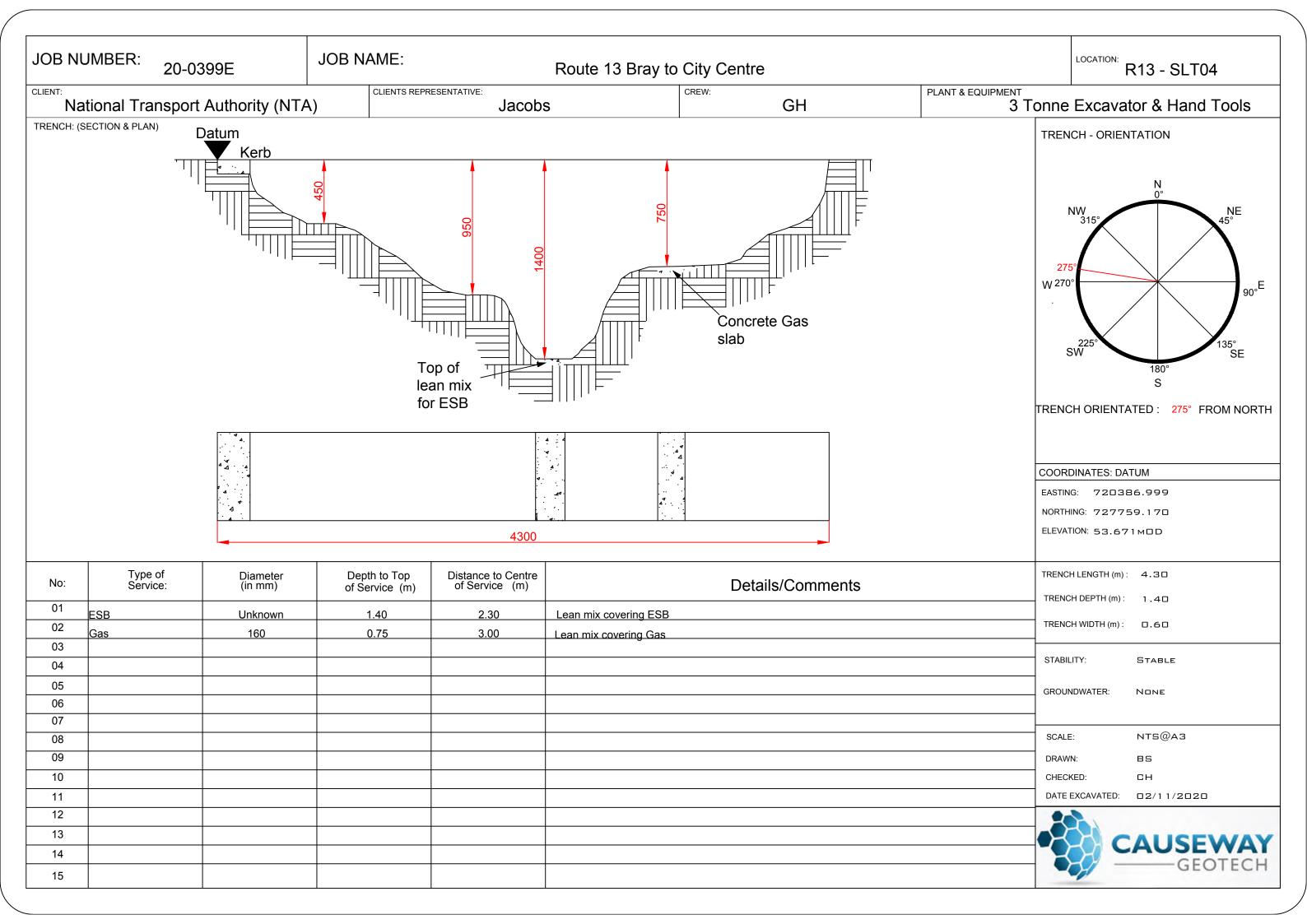


0-				
05				
06				
07				
08				
09				
10				
11				
12				
13				
14				
15				
	1	1	1	

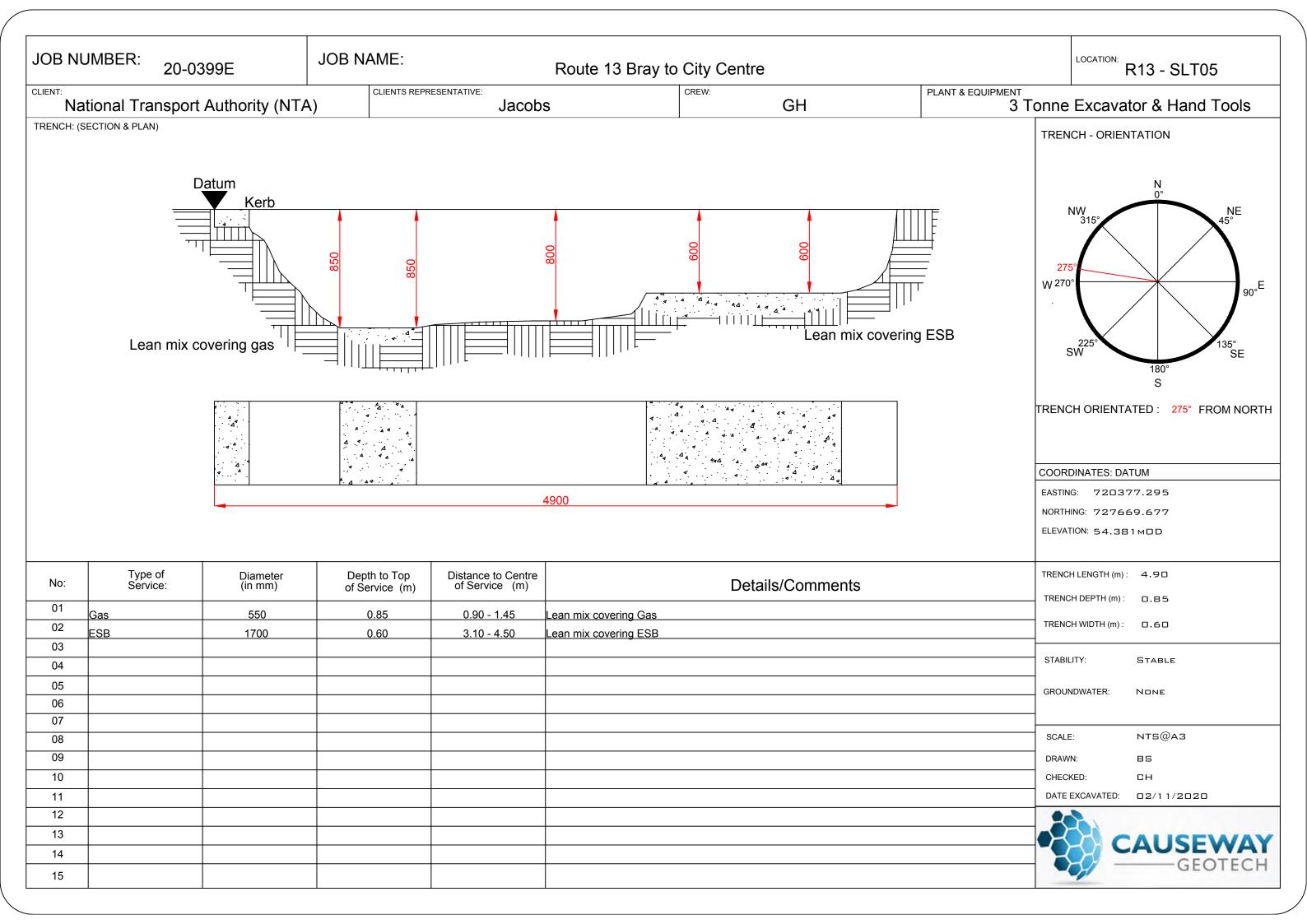
	CAUS	ΈΨΔΥ		ect No. 0399E	Bus Co	Name: nnects Route 13 Bray to City Centre			rial Pit ID		
	G	EWAY EOTECH		dinates 36.49 E	Client: National Transport Authority (NTA)				R13-SLT03A		
Method:				80.61 N		Representative:		S	heet 1 of 1		
Slit Trenching					Jacobs		Logger:		Scale: 1:25		
Plant:			Elevation		Date:	FINAL					
3T Tracked Exc				3 mOD	20/10/	2020	MG				
Depth (m)	Sample / Tests	Field Records	Level (mOD)	Depth (m)	Legend	Description		Water			
			47.73	0.20		MADE GROUND: Soft slightly sandy slightly silty CLJ cobble content, fragments of brick, concrete, tin, ai to coarse. Gravel is angular to subangular fine to co lithologies. Cobbles are subangular of limestone.	nd plastic. Sand is fine		0.5 -		
			47.11	- 0.82 - - -		End of trial pit at 0.82m			1.0		
				-					1.5 —		
				-					-		
				- - - - -					2.0		
				-							
				-					2.5 -		
				- - - - - - -					3.0		
				- - - - - - -					3.5 -		
				- - - - - - -					4.0		
				- - - -							
				- - - - - - -					4.5 -		
	Strikes	Depth: 0.82	Rema	arks:	encountor	ed					
Struck at (m)	Remarks	Width: 0.58 Length: 3.78	Old ro Manh Old ro	oad level en Iole cover e Dad curbs/p	countered ncountere avers enco	ed. at 0.27mbgl. d at 0.34mbgl. puntered at 0.60-0.82mbgl.					
		Stability: Stable		ination Re		action of the engineer.	Last Updated 17/12/2020		AGS		



GEOTECH			20-0	ect No. 0399E		: Name: nnects Route 13 Bray to City Centre		Trial Pit ID R13-SLT04		
Method: Slit Trenching	G	EOTECH	- 7203	dinates 87.00 E 59.17 N	National Transport Authority (NTA) Client's Representative: Jacobs				heet 1 of 1 Scale: 1:25	
Plant:				vation	Date:	Logger:		FINAL		
3T Tracked Ex Depth	cavator Sample /		53.67 Level	7 mOD Depth	02/11/		GH	5	FINAL	
(m)	Tests	Field Records	(mOD)	(m)	Legend	Description		Water		
0.50 0.50	B1 B2		53.47	0.20		MADE GROUND: Firm brown slightly sandy slightly cobble content. Sand is fine to coarse. Gravel is sub of mixed lithologies. Cobbles are subrounded of mi	angular fine to coarse		0.5	
			52.77	0.90		MADE GROUND: Brown fine to coarse SAND.			1.0	
			52.27	1.40		End of trial pit at 1.40m				
				-					2.0	
				-					-	
				-					2.5	
				- - - - - -					3.0	
				- - - - -					-	
									3.5 —	
				- - - - -					4.0	
				- - - - -						
				- - - - - -					-	
Wate Struck at (m)	er Strikes Remarks	Depth: 1.40 Width: 0.60 Length: 4.30	Rema No gr	arks: oundwate	er encour	tered.				
		Stability: Stable		ination Re		uction of the engineer.	Last Updated 17/12/2020		AGS	

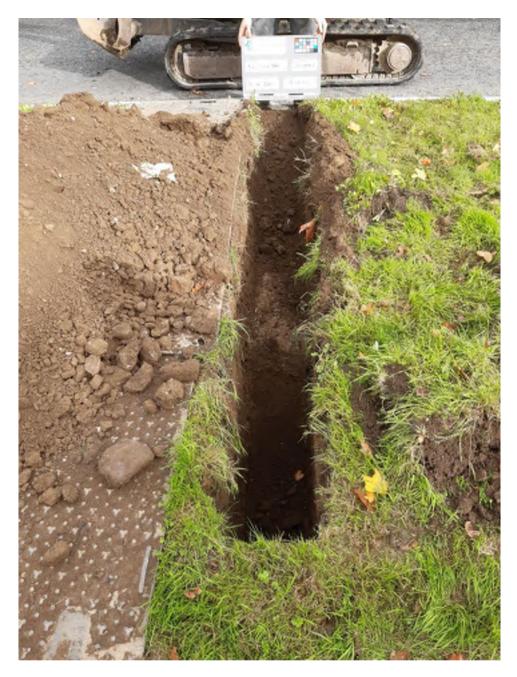


CAUSEWAY GEOTECH			Project No. 20-0399E Coordinates 720377.30 E 727669.68 N Elevation 54.38 mOD		Project Name: Bus Connects Route 13 Bray to City Centre Client: National Transport Authority (NTA) Client's Representative: Jacobs Date: 02/11/2020 GH			Trial Pit ID R13-SLT05	
Slit Trenching Plant:									
Plant: 3T Tracked Excavator								FINAL	
Depth	Sample /							Level	Depth
(m)	Tests	Field Records	(mOD)	(m)	Legend	Description		Water	
			54.18	0.20		MADE GROUND: Firm brown slightly sandy slightly cobble content. Sand is fine to coarse. Gravel is sub of mixed lithologies. Cobbles are subrounded of mi	angular fine to coarse		
			53.78	0.60		MADE GROUND: Brown fine to coarse SAND.			0.5 -
			53.53	- 0.85 -		End of trial pit at 0.85m			1.0
				-					-
				-					1.5 —
				-					-
				-					2.0
				-					-
				-					2.5 —
				-					
				- - - 					3.0
				- - - -					-
				-					3.5 —
				-					-
				- - -					4.0
				-					-
				- - - -					4.5 -
				-					-
				-					
Water Struck at (m)	r Strikes Remarks	Depth: 0.85 Width: 0.60 Length: 4.90		Remarks: No groundwater encountered.					
Stability: Stable				Termination Reason: Last Updated Terminated on the instruction of the engineer. 17/12/2020				AGS	



APPENDIX D SLIT TRENCH PHOTOGRAPHS

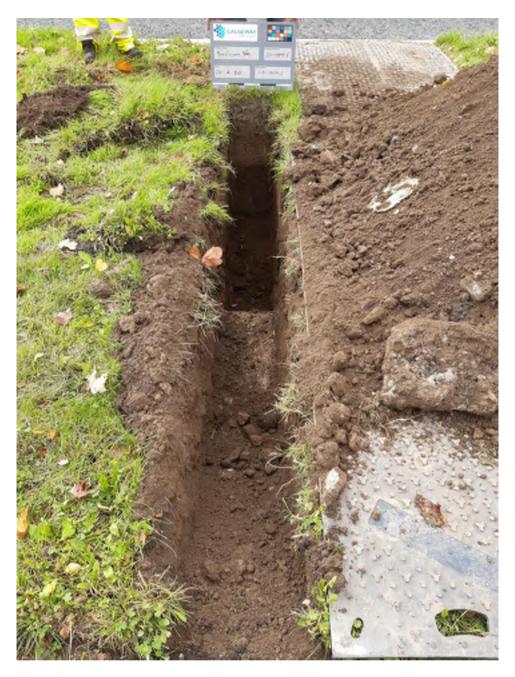
Report No.: 20-0399E



R13-SLT01



Report No.: 20-0399E



R13-SLT01



Report No.: 20-0399E



R13-SLT01





Report No.: 20-0399E



R13-SLT01





Report No.: 20-0399E





Report No.: 20-0399E



R13-SLT02



Report No.: 20-0399E



R13-SLT02



Report No.: 20-0399E



R13-SLT02





Report No.: 20-0399E



R13-SLT02





Report No.: 20-0399E



R13-SLT02





Report No.: 20-0399E



R13-SLT03



Report No.: 20-0399E



R13-SLT03



Report No.: 20-0399E



R13-SLT03



R13-SLT03



Report No.: 20-0399E





Report No.: 20-0399E



R13-SLT03



Report No.: 20-0399E



R13-SLT03A



Report No.: 20-0399E



R13-SLT03A



Report No.: 20-0399E



R13-SLT03A



Report No.: 20-0399E



R13-SLT03A



Report No.: 20-0399E



R13-SLT03A



R13-SLT03A



Report No.: 20-0399E



R13-SLT03A



R13-SLT03A



Report No.: 20-0399E





Report No.: 20-0399E



R13-SLT04



Report No.: 20-0399E





Report No.: 20-0399E



R13-SLT04



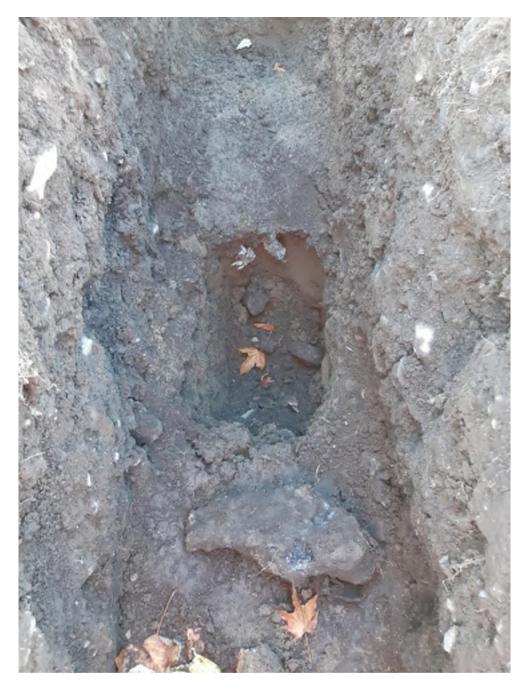
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R13-SLT04



Report No.: 20-0399E



R13-SLT04



Report No.: 20-0399E



R13-SLT04



Report No.: 20-0399E



R13-SLT04



Report No.: 20-0399E



R13-SLT04



Report No.: 20-0399E



R13-SLT04



R13-SLT04

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Report No.: 20-0399E





Report No.: 20-0399E



R13-SLT05



Report No.: 20-0399E



R13-SLT05



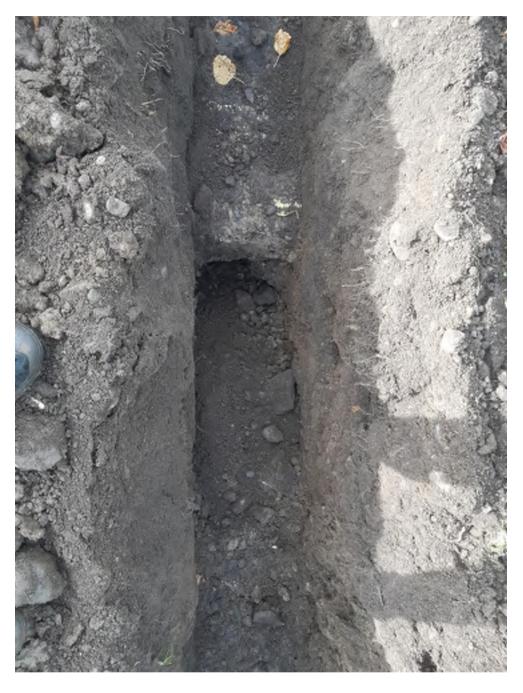
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R13-SLT05



Report No.: 20-0399E



R13-SLT05



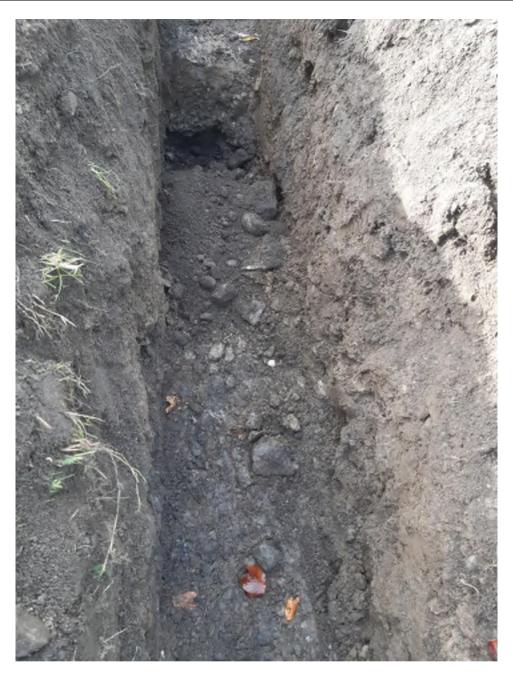
Report No.: 20-0399E



R13-SLT05



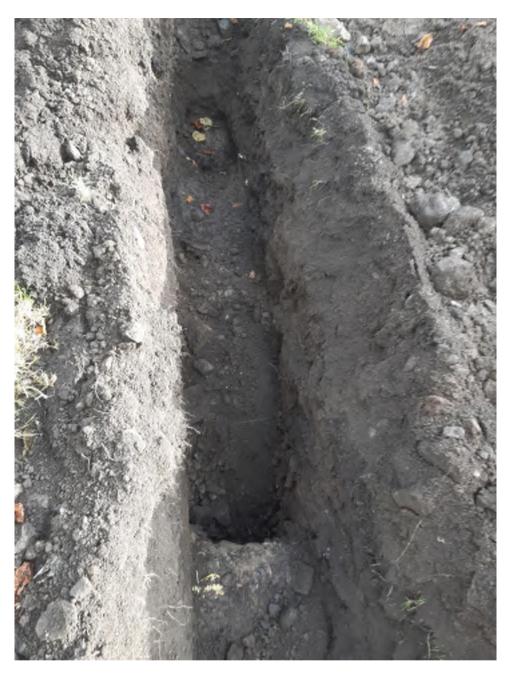
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R13-SLT05



Report No.: 20-0399E



R13-SLT05



Report No.: 20-0399E



R13-SLT05



R13-SLT05



Report No.: 20-0399E



R13-SLT05



APPENDIX E GEOTECHNICAL LABORATORY TEST RESULTS



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Registered in Northern Ireland.

Company Number: NI610756

REGIONAL OFFICE Causeway Geotech (IRL) Ltd

Unit 3 Balbriggan Business Park, Balbriggan Co Dublin, Ireland, K32 EH36 ROI: +353 (0)1 526 7465

> Registered in Ireland. Company Number: 633786

www.causewaygeotech.com

SOIL AND ROCK SAMPLE ANALYSIS LABORATORY TEST REPORT

19 November 2020

Project Name:	Bus Connects - Route 13 – Bray to City Centre
Project No.:	20-0399E
Client:	National Transport Authority (NTA)
Engineer:	AECOM

We are pleased to attach the results of laboratory testing carried out for the above project. This memo and its attachments constitute a report of the results of tests as detailed in the Contents page(s).

The attached results complete the testing requested and we would therefore wish to confirm that samples will be retained without charge for a period of 28 days from the above date after which they will be appropriately disposed of unless we receive written instructions to the contrary prior to that date.

We trust our report meets with your approval but if you have any queries or require additional information, please do not hesitate to contact the undersigned.

John Wom

Stephen Watson Laboratory Manager Signed for and on behalf of Causeway Geotech Ltd













Project Name: Bus Connects - Route 13 – Bray to City Centre

Report Reference: Schedule 1

The table below details the tests carried out, the specifications used, and the number of tests included in this report.

Tests marked with* in this report are not United Kingdom Accreditation Service (UKAS) accredited and are not included in Causeway Geotech Limited's scope of UKAS Accreditation Schedule of Tests. Opinions and interpretations expressed herein are outside the scope of UKAS accreditation.

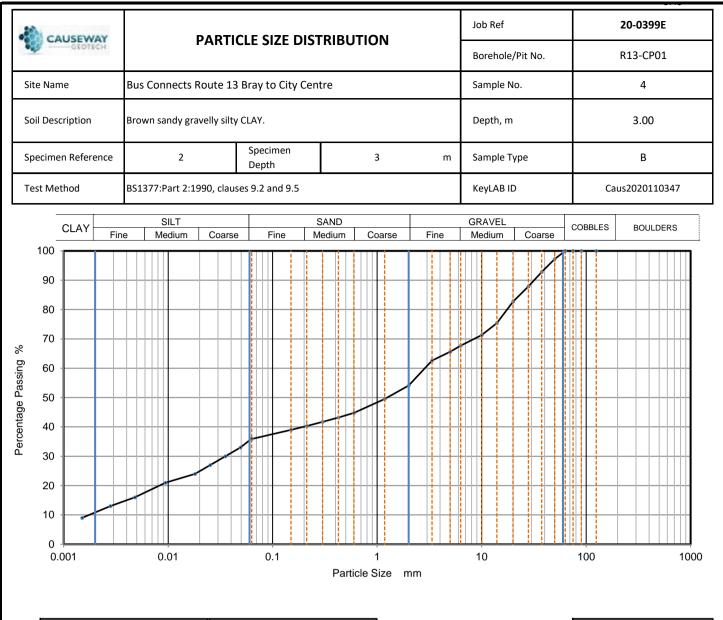
Material tested	Type of test/Properties measured/Range of measurement	Standard specifications	No. of results included in the report
SOIL	Moisture Content of Soil	BS 1377-2: 1990: Cl 3.2	9
SOIL	Liquid and Plastic Limits of soil-1 point cone penetrometer method	BS 1377-2: 1990: Cl 4.4, 5.3 & 5.4	2
SOIL	Particle size distribution - wet sieving	BS 1377-2: 1990: Cl 9.2	9
SOIL	Particle size distribution - sedimentation hydrometer method	BS 1377-2: 1990: Cl 9.5	6

SUB-CONTRACTED TESTS

In agreement with Client, the following tests were conducted by an approved sub-contractor. All subcontracting laboratories used are UKAS accredited.

Material tested	Type of test/Properties measured/Range of measurement	Standard specifications	No. of results included in the report
SOIL – Subcontracted to Eurofins Chemtest Ltd <i>(UKAS</i> 2183)	BRE Test – Suite B		4

	JSEN GEO	NAY						sification Test Results						
Project No.			Project	Name										
20-03	899E				Bus	Connec	ts Ro	ute 13	Bray to	City Cen	tre			
Hole No.			nple		Soil Description	Dens bulk	ity dry	W	Passing 425µm	LL	PL	PI	Particle density	Casagrande
	Ref	Тор	Base	Туре		Mg/m	 13	%	%	%	%	%	Mg/m3	Classification
R13-CP01	3	2.00		В	Brown sandy gravelly silty CLAY.			14.0	64	32 -1pt	17	15		CL
R13-CP01	13	3.00		D	Brown sandy gravelly silty CLAY.			14.0						
R13-CP01	14	4.50		D	Brown sandy gravelly silty CLAY.			16.0						
R13-CP02	9	1.20		D	Grey gravelly clayey subangular fine to coarse GRAVEL.			7.5						
R13-CP02	3	2.00		В	Brown sandy very gravelly silty CLAY.			7.6						
R13-CP03	16	2.00		D	Brown sandy gravelly silty CLAY.			22.0	58	36 -1pt	22	14		CI
R13-CP03	17	4.00		D	Brown sandy silty CLAY.			17.0						
R13-CP03	7	6.00		в	Brown gravelly slightly clayey fine to coarse SAND.			7.3						
R13-WS01	5	1.30		В	Brown gravelly clayey fine to coarse SAND.			6.4						
All tests perfor	med i	n accord	ance wit	h BS1	377:1990 unless specified	otherwis	e			-			LAB	01R Version 4
	neasure	ment unles	s :		ie unless : sp - sn	e density nall pyknom	eter	Date F	Printed	020	Appr	oved	Ву	
wd - wai wi - imn		acement in water			asagrande method gj - ga: ngle point test	s jar					Step	hen.	Watson	UKAS TESTING 10122



Siev	ing	Sedimentation				
Particle Size mm	% Passing	Particle Size mm	% Passing			
125	100	0.06300	36			
90	100	0.04921	33			
75	100	0.03524	30			
63	100	0.02523	27			
50	97	0.01806	24			
37.5	93	0.00944	21			
28	88	0.00480	16			
20	83	0.00280	13			
14	75	0.00150	9			
10	71					
6.3	68					
5	66					
3.35	63					
2	54					
1.18	50					
0.6	45	Particle density	(assumed)			
0.425	43	2.65	Mg/m3			
0.3	42					
0.212	40					
0.15	39					
0.063	36					

Dry Mass of sample, g

6625

Sample Proportions	% dry mass
Cobbles	0.0
Gravel	45.9
Sand	18.1
Silt	25.0
Clay	11.0

Grading Analysis		
D100	mm	
D60	mm	2.86
D30	mm	0.0356
D10	mm	0.00173
Uniformity Coefficient		1700
Curvature Coefficient		0.26

Remarks

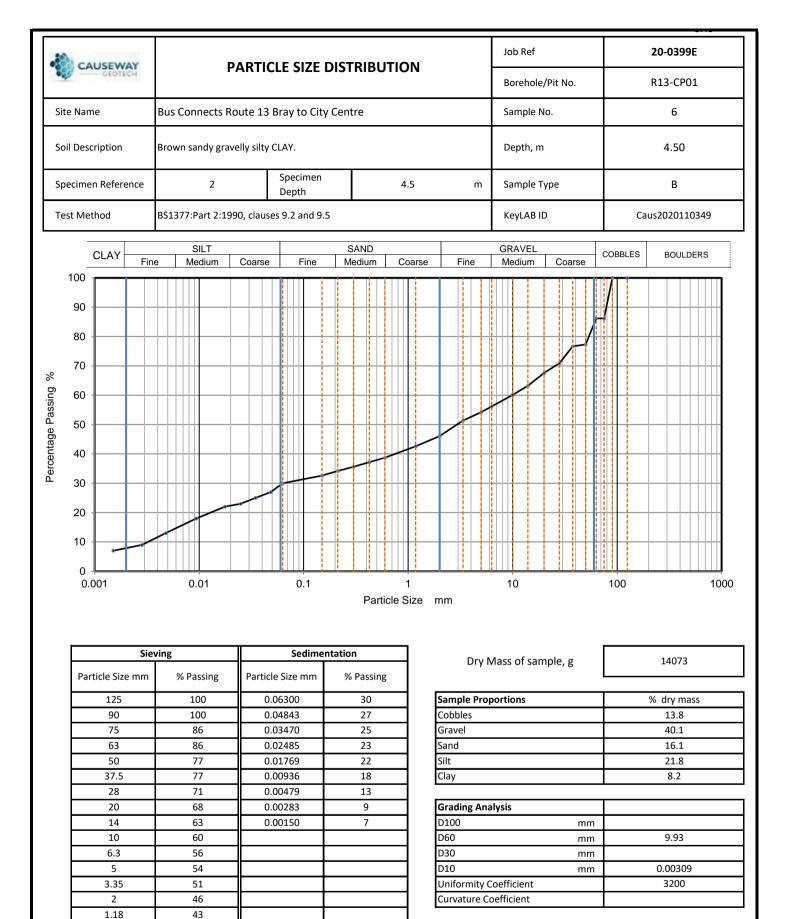
Preparation and testing in accordance with BS1377-2 :1990 unless noted below



Approved

Stephen.Watson

LAB 05R Version 4



Remarks

Preparation and testing in accordance with BS1377-2 :1990 unless noted below



Approved

39

37

36

34

33

30

Particle density

2.65

(assumed)

Mg/m3

0.6

0.425

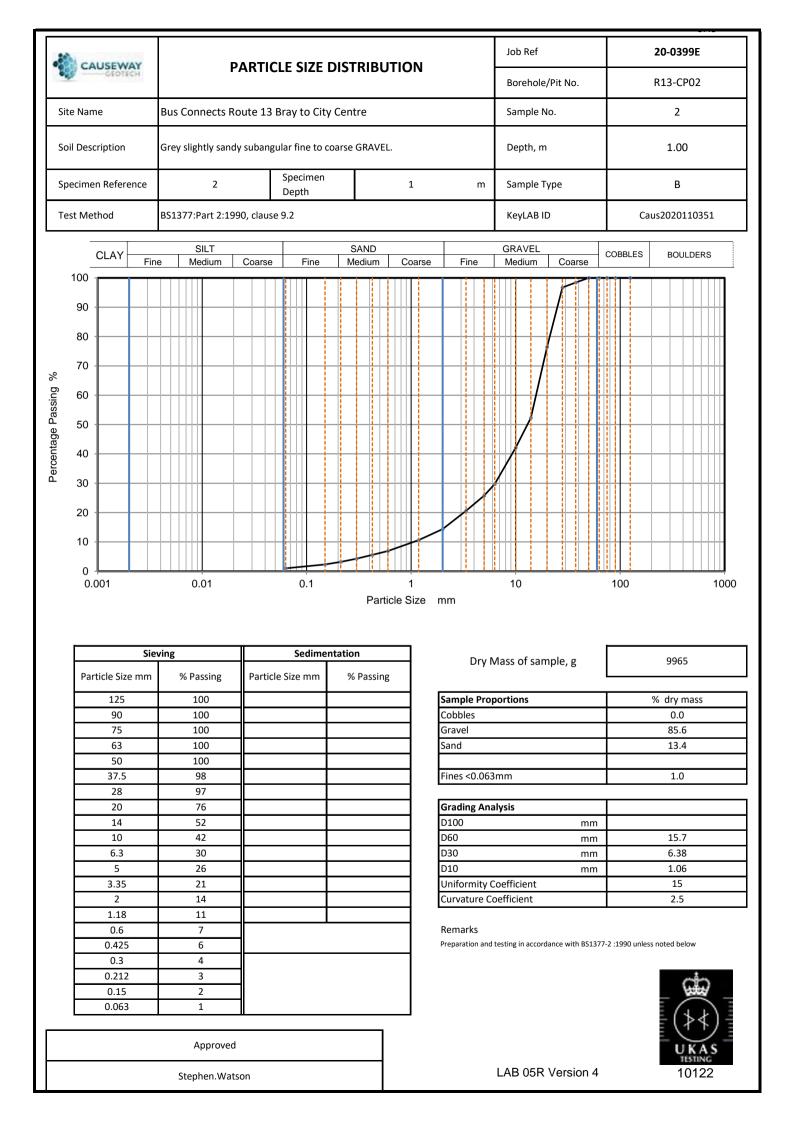
0.3

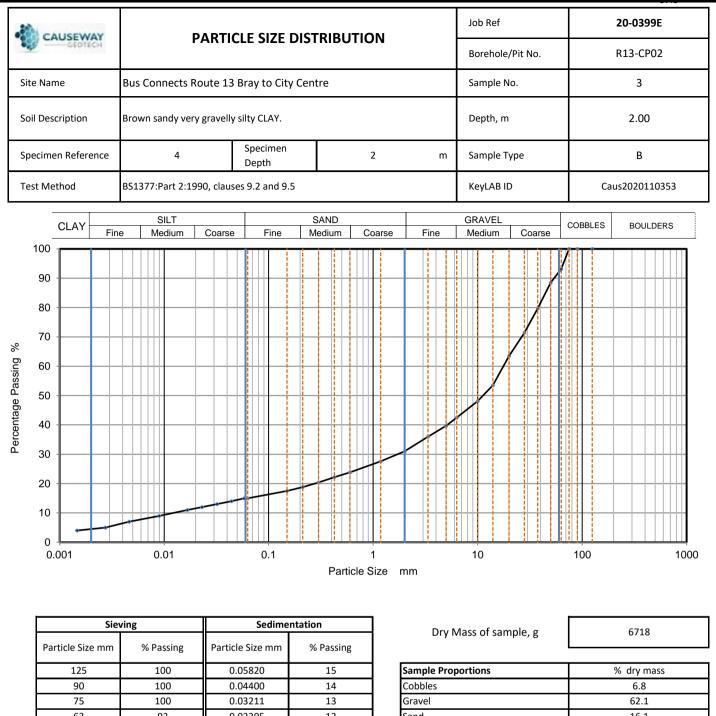
0.15

0.063

Stephen.Watson

LAB 05R Version 4





Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.05820	15
90	100	0.04400	14
75	100	0.03211	13
63	93	0.02305	12
50	89	0.01666	11
37.5	80	0.00896	9
28	71	0.00462	7
20	64	0.00273	5
14	54	0.00146	4
10	48		
6.3	43		
5	40		
3.35	36		
2	31		
1.18	28		
0.6	24	Particle density	(assumed)
0.425	22	2.65	Mg/m3
0.3	20		
0.212	19		
0.15	18		
0.063	15		

Sample Proportions	% dry mass
Cobbles	6.8
Gravel	62.1
Sand	16.1
Silt	10.1
Clay	4.9

Grading Analysis		
D100	mm	
D60	mm	17.7
D30	mm	1.71
D10	mm	0.0126
Uniformity Coefficient		1400
Curvature Coefficient		13

Remarks

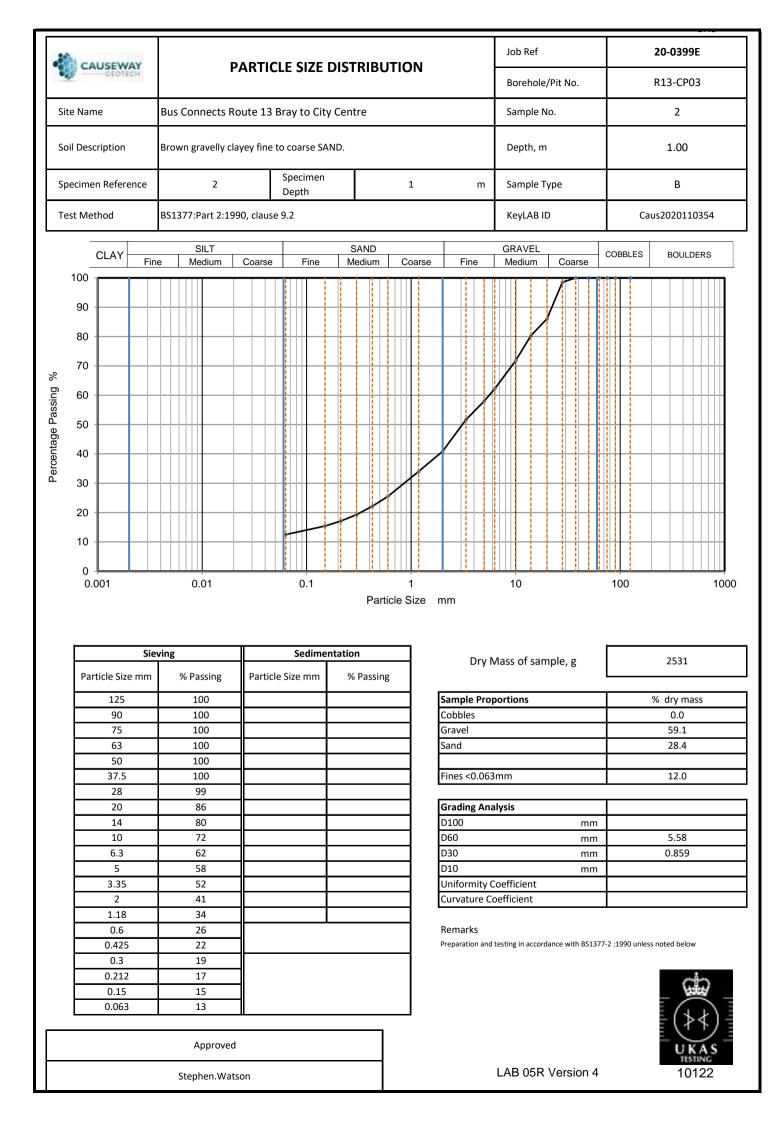
Preparation and testing in accordance with BS1377-2 :1990 unless noted below



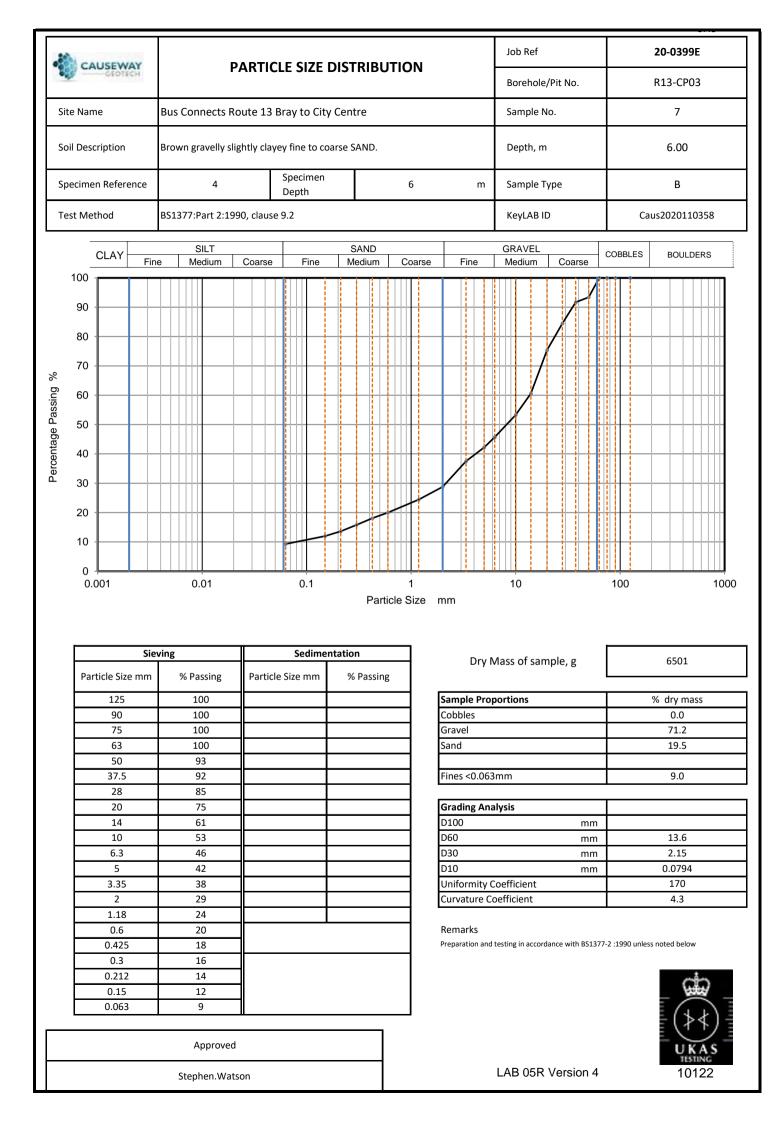
Approved

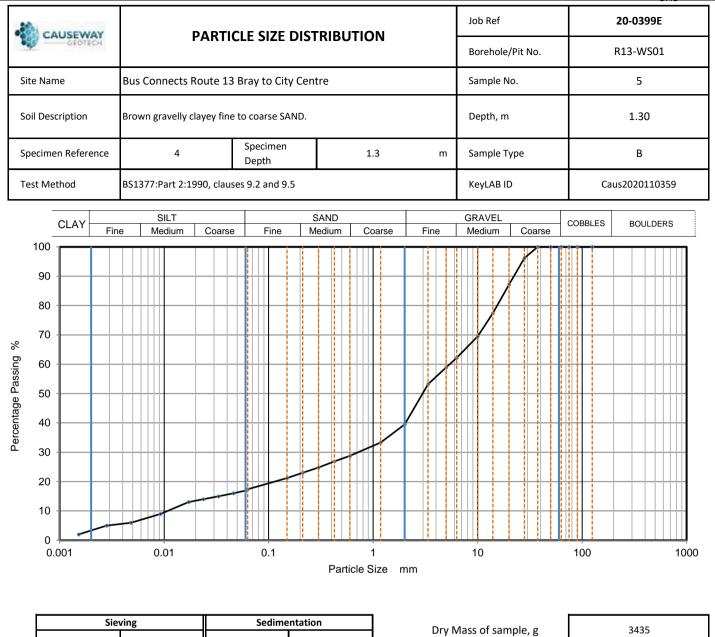
Stephen.Watson

LAB 05R Version 4



THE	CAUSEWAY		PARTICLE SIZ			ı	Job Ref			20-0399E
	GEOTECH		PARTICLE SIZ			V	Borehole/	Pit No.		R13-CP03
Site Na	ame	Bus Connects F	Route 13 Bray to (City Centre	2		Sample No.		5	
Soil De	escription	n Brown sandy silty CLAY.				Depth, m			4.00	
Specim	men Reference	2	2 Specimen 4 m Depth			m	Sample Ty	vpe		В
Test M	/lethod	BS1377:Part 2:19	990, clauses 9.2 and	9.5			KeyLAB ID	I	Ca	aus2020110356
	CLAY	SILT Fine Medium	Coarse Fine	SAN Medi		rse Fine	GRAVEL Medium	Coarse	COBBLES	BOULDERS
100	0									
90	0									
80	0									
70	0									
2										
ő 60	0									
60 50 40	0									
40	0									
; · 30										
00										
20	0									
10										
TU										
					1 Particle Siz	e mm	10		100	100
					Particle Siz	e mm	10		100	100
	s	ileving	Sedir	nentation	-			ple, g	100	216
P	Sparticle Size mr		Sedir Particle Size mn	nentation	-		10 Mass of sam	ple, g	100	
P	Particle Size mr	n % Passing	Particle Size mn	nentation	Particle Siz	Dry M	Mass of sam	ple, g	100	216
P				nentation	Particle Siz		Mass of sam	ple, g		
P	Particle Size mr 125 90 75	n % Passing 100 100 100	Particle Size mn 0.06300 0.04104 0.02929	nentation n % Pa	Particle Siz	Dry N Sample Pro Cobbles Gravel	Mass of sam	ple, g		216 % dry mass 0.0 5.3
P	Particle Size mr 125 90 75 63	n % Passing 100 100 100 100	Particle Size mn 0.06300 0.04104 0.02929 0.02109	nentation h % Pa	Particle Siz	Dry N Sample Pro Cobbles Gravel Sand	Mass of sam	ple, g		216 % dry mass 0.0 5.3 23.5
P	Particle Size mr 125 90 75	n % Passing 100 100 100	Particle Size mn 0.06300 0.04104 0.02929	nentation h % Pa	Particle Siz	Dry N Sample Prop Cobbles Gravel Sand Silt	Mass of sam	ple, g		216 % dry mass 0.0 5.3
P	Particle Size mr 125 90 75 63 50	n % Passing 100 100 100 100 100	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543	nentation % Pa 6 6 6 6 6 6 6 6 6 6 6 6 6	Particle Siz	Dry N Sample Pro Cobbles Gravel Sand	Mass of sam	ple, g		216 % dry mass 0.0 5.3 23.5 40.3
P	Particle Size mr 125 90 75 63 50 37.5 28 20	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260	nentation % Pa 6 6 6 6 6 6 6 6 6 6 6 6 6	Particle Siz	Dry N Sample Prop Cobbles Gravel Sand Silt Clay Grading Ana	Mass of sam	ple, g		216 % dry mass 0.0 5.3 23.5 40.3
P	Particle Size mr 125 90 75 63 50 37.5 28 20 14	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439	nentation % Pa 6 6 6 6 6 6 6 6 6 6 6 6 6	Particle Siz	Dry N Sample Pro Cobbles Gravel Sand Silt Clay Grading Ana D100	Mass of sam	mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9
P	Particle Size mr 125 90 75 63 50 37.5 28 20 14 10	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260	nentation % Pa 6 6 6 6 6 6 6 6 6 6 6 6 6	Particle Siz	Dry N Sample Pro Cobbles Gravel Sand Silt Clay Grading Ana D100 D60	Mass of sam	mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9 0.0171
P	Particle Size mr 125 90 75 63 50 37.5 28 20 14	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260	nentation % Pa 6 6 6 6 6 6 6 6 6 6 6 6 6	Particle Siz	Dry N Sample Pro Cobbles Gravel Sand Silt Clay Grading Ana D100	Mass of sam	mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9
	Particle Size mr 125 90 75 63 50 37.5 28 20 14 10 6.3 5 3.35	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260	nentation % Pa 6 6 6 6 6 6 6 6 6 6 6 6 6	Particle Siz	Dry N Sample Prop Cobbles Gravel Sand Silt Clay Clay Grading Ana D100 D60 D30 D10 Uniformity C	Vlass of sam	mm mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9 0.0171
	Particle Size mr 125 90 75 63 50 37.5 28 20 14 10 6.3 5 3.35 2	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260	nentation % Pa 6 6 6 6 6 6 6 6 6 6 6 6 6	Particle Siz	Dry N Sample Proj Cobbles Gravel Sand Silt Clay D100 D60 D30 D10	Vlass of sam	mm mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9 0.0171
	Particle Size mr 125 90 75 63 50 37.5 28 20 14 10 6.3 5 3.35 2 1.18	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260 0.00140	nentation % Pa () () () () () () () () () ()	Particle Siz	Dry N Sample Proj Cobbles Gravel Sand Silt Clay Grading Ana D100 D60 D30 D10 Uniformity C Curvature C	Vlass of sam	mm mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9 0.0171
	Particle Size mr 125 90 75 63 50 37.5 28 20 14 10 6.3 5 3.35 2	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260	nentation % Pa () () () () () () () () () ()	Particle Siz	Dry N Sample Proj Cobbles Gravel Sand Silt Clay Grading Ana D100 D60 D30 D10 Uniformity C Curvature Co Remarks	Vlass of sam	mm mm mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9 0.0171 0.00181
	Particle Size mr 125 90 75 63 50 37.5 28 20 14 10 6.3 5 3.35 2 1.18 0.6	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260 0.00140	nentation % Pa	Particle Siz	Dry N Sample Proj Cobbles Gravel Sand Silt Clay Grading Ana D100 D60 D30 D10 Uniformity C Curvature Co Remarks	Mass of sam	mm mm mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9 0.0171 0.00181
	Particle Size mr 125 90 75 63 50 37.5 28 20 14 10 6.3 5 3.35 2 1.18 0.6 0.425 0.3 0.212	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260 0.00140	nentation % Pa	Particle Siz	Dry N Sample Proj Cobbles Gravel Sand Silt Clay Grading Ana D100 D60 D30 D10 Uniformity C Curvature Co Remarks	Mass of sam	mm mm mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9 0.0171 0.00181
	Particle Size mr 125 90 75 63 50 37.5 28 20 14 10 6.3 5 3.35 2 1.18 0.6 0.425 0.3 0.212 0.15	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260 0.00140	nentation % Pa	Particle Siz	Dry N Sample Proj Cobbles Gravel Sand Silt Clay Grading Ana D100 D60 D30 D10 Uniformity C Curvature Co Remarks	Mass of sam	mm mm mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9 0.0171 0.00181
	Particle Size mr 125 90 75 63 50 37.5 28 20 14 10 6.3 5 3.35 2 1.18 0.6 0.425 0.3 0.212	n % Passing 100 100 100 100 100 100 100 10	Particle Size mn 0.06300 0.04104 0.02929 0.02109 0.01543 0.00835 0.00439 0.00260 0.00140 Particle density 2.65	nentation % Pa	Particle Siz	Dry N Sample Proj Cobbles Gravel Sand Silt Clay Grading Ana D100 D60 D30 D10 Uniformity C Curvature Co Remarks	Mass of sam	mm mm mm		216 % dry mass 0.0 5.3 23.5 40.3 30.9 0.0171 0.00181





Siev	ing	Sedimentation		
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100	0.06107	17	
90	100	0.04609	16	
75	100	0.03307	15	
63	100	0.02372	14	
50	100	0.01712	13	
37.5	100	0.00925	9	
28	96	0.00482	6	
20	87	0.00281	5	
14	77	0.00152	2	
10	70			
6.3	62			
5	59			
3.35	53			
2	40			
1.18	33			
0.6	29	Particle density	(assumed)	
0.425	27	2.65	Mg/m3	
0.3	25			
0.212	23			
0.15	21	71		
0.063	17			

Sample Proportions	% dry mass
Cobbles	0.0
Gravel	60.5
Sand	22.0
Silt	13.9
Clay	3.6

Grading Analysis		
D100	mm	
D60	mm	5.41
D30	mm	0.721
D10	mm	0.0102
Uniformity Coefficient		530
Curvature Coefficient		9.4

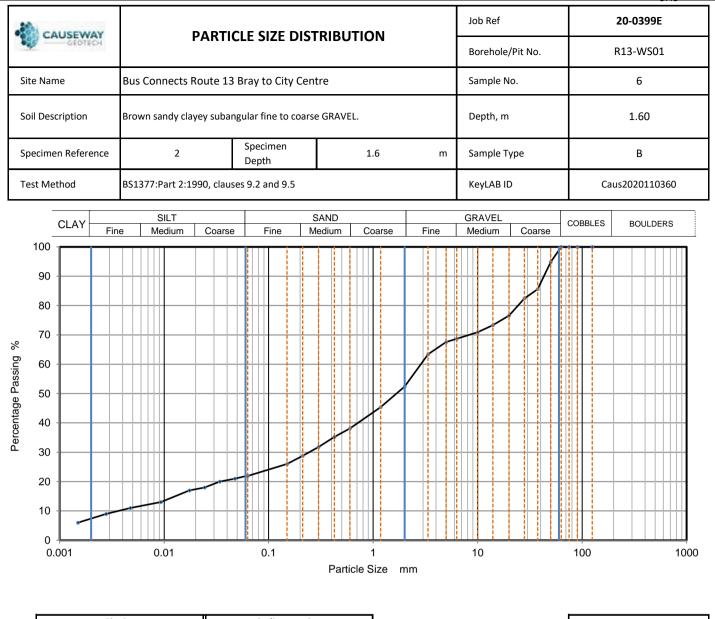
Remarks

Preparation and testing in accordance with BS1377-2 :1990 unless noted below



Approved

Stephen.Watson



Siev	ing	Sedim	entation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.06300	22
90	100	0.04744	21
75	100	0.03401	20
63	100	0.02437	18
50	95	0.01746	17
37.5	86	0.00930	13
28	82	0.00474	11
20	77	0.00278	9
14	73	0.00150	6
10	71		
6.3	69		
5	68		
3.35	63		
2	52		
1.18	45		
0.6	38	Particle density	(assumed)
0.425	35	2.65	Mg/m3
0.3	32		
0.212	29		
0.15	26		
0.063	22		

Dry Mass of sample, g

5323

Sample Proportions	% dry mass
Cobbles	0.0
Gravel	47.6
Sand	30.5
Silt	14.7
Clay	7.2

Grading Analysis		
D100	mm	
D60	mm	2.86
D30	mm	0.243
D10	mm	0.00376
Uniformity Coefficient		760
Curvature Coefficient		5.5

Remarks

Preparation and testing in accordance with BS1377-2 :1990 unless noted below



Approved

Stephen.Watson

😵 eurofins



Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL Tel: 01638 606070 Email: info@chemtest.com

Final Report			Email: info@chemtest.com
Report No.:	20-30179-1		
Initial Date of Issue:	11-Nov-2020		
Client	Causeway Geotech Ltd		
Client Address:	8 Drumahiskey Road Balnamore Ballymoney County Antrim BT53 7QL		
Contact(s):	Carin Cornwall Colm Hurley Darren O'Mahony Gabriella Horan Joe Gervin John Cameron Lucy Newland Martin Gardiner Matthew Gilbert Neil Haggan Paul Dunlop Sean Ross Stephen Franey Stephen McCracken Stephen Watson Stuart Abraham Thomas McAllis		
Project	20-0399E Route 13 Bray to City Centre		
Quotation No.:		Date Received:	06-Nov-2020
Order No.:		Date Instructed	l: 06-Nov-2020
No. of Samples:	4		
Turnaround (Wkdays):	5	Results Due:	12-Nov-2020
Date Approved:	11-Nov-2020		
Approved By:			
Ulfa Par			
Details:	Glynn Harvey, Technical Manager		

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Chemtest

Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL Tel: 01638 606070 Email: info@chemtest.com

<u>Results - Soil</u>

Project: 20-0399E Route 13 Bray to City Centre

Client: Causeway Geotech Ltd	Chemtest Job No.:			20-30179	20-30179	20-30179	20-30179	
Quotation No.:	(Chemtest Sample ID.:		1092939	1092940	1092941	1092942	
Order No.:		Clie	nt Samp	le Ref.:	9	14	2	20
		Sa	ample Lo	ocation:	R13-CP01	R13-CP01	R13-CP02	R13-CP03
			Sampl	e Type:	SOIL	SOIL	SOIL	SOIL
		Top Depth (m):		2.00	4.50	1.00	3.00	
			Date Sa	ampled:	05-Nov-2020	05-Nov-2020	05-Nov-2020	05-Nov-2020
Determinand	Accred.	SOP	Units	LOD				
Moisture	Ν	2030	%	0.020	13	10	6.9	14
pH (2.5:1)	Ν	2010		4.0	8.7	8.8	10.8	8.8
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	< 0.010	0.025	0.51	< 0.010
Total Sulphur	U	2175	%	0.010	< 0.010	0.046	0.14	
Sulphate (Acid Soluble)	U	2430	%	0.010	0.035	0.030	0.25	

Test Methods

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2040	Soil Description(Requirement of MCERTS)	Soil description	As received soil is described based upon BS5930
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2175	Total Sulphur in Soils	Total Sulphur	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.

Report Information

Key

1.09	
U	UKAS accredited
Μ	MCERTS and UKAS accredited
Ν	Unaccredited
S	This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
SN	This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
Т	This analysis has been subcontracted to an unaccredited laboratory
I/S	Insufficient Sample
U/S	Unsuitable Sample
N/E	not evaluated
<	"less than"
>	"greater than"
	Comments or interpretations are beyond the scope of UKAS accreditation
	The results relate only to the items tested
	Uncertainty of measurement for the determinands tested are available upon request
	None of the results in this report have been recovery corrected
	All results are expressed on a dry weight basis
	The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis All Asbestos testing is performed at the indicated laboratory Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

Sample Deviation Codes

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

Sample Retention and Disposal

All soil samples will be retained for a period of 45 days from the date of receipt All water samples will be retained for 14 days from the date of receipt Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>



LABORATORY RESTRICTION REPORT

Project Reference	20-0399E			То	Sean Ross
Project Name	Bus Connects Route 9 - Bray to City Centre			Position	Project Manager
i lojeet Name	Bus Connects Route 9 - Bray to City Centre			From	Joseph Nicholl
	00.00005			e coopii i dononi	
TR reference	20-0399E	20-0399E / G01		Position	Laboratory Quality Manager

The following sample(s) and test(s) are restricted as detailed below. Could you please complete the "Required Action" column and return the completed form to the laboratory.

Hole		Sample		Test		
Number	Number	Depth	Туре	Туре	Reason for Restriction	Required Action
R13 CP01	15	(m) 3.00	U	UU Triaxial Oedometer	Unable to obtain specimen for test - coarse gravel content too high	CANCEL
For electr	onic repor	ting a forn	n of I name	eis	Laboratory Signature Joseph Nicholl	Project Manager Signature Sean Ross
acceptable					Date 13 November 2020	Date

APPENDIX F ENVIRONMENTAL LABORATORY TEST RESULTS

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Chemtest

Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL Tel: 01638 606070 Email: info@chemtest.com

Report No.:	20-28443-1		
Initial Date of Issue:	26-Oct-2020		
Client	Causeway Geotech Ltd		
Client Address:	8 Drumahiskey Road Balnamore Ballymoney County Antrim BT53 7QL		
Contact(s):	Carin Cornwall Colm Hurley Darren O'Mahony Gabriella Horan Joe Gervin John Cameron Lucy Newland Martin Gardiner Matthew Gilbert Neil Haggan Paul Dunlop Sean Ross Stephen Franey Stephen McCracken Stephen Watson Stuart Abraham Thomas McAllis		
Project	20-0399E Bus Connects Route 13		
Quotation No.:		Date Received:	21-Oct-2020
Order No.:		Date Instructed:	21-Oct-2020
No. of Samples:	1		
Turnaround (Wkdays):	5	Results Due:	27-Oct-2020
Date Approved:	26-Oct-2020		
Approved By:			
Mar Mary			
Details:	Glynn Harvey, Technical Manager		

THE ENVIRONMENT

Final Report

2183

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Chemtest

Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL Tel: 01638 606070 Email: info@chemtest.com Project: 20-0399E Bus Connects Route 13

Client: Causeway Geotech Ltd	_	Che	mtest Jo	ob No.:	20-28443
Quotation No.:	0	Chemte	st Sam	ple ID.:	1084161
		Sa	ample Lo	cation:	R13-CP03
	Sample Type:		SOIL		
			Top Dep	oth (m):	1.0
			Date Sa	ampled:	19-Oct-2020
			Asbest	os Lab:	DURHAM
Determinand	Accred.	SOP	Units	LOD	
АСМ Туре	U	2192		N/A	-
Asbestos Identification	U	2192		N/A	No Asbestos Detected
ACM Detection Stage	U	2192		N/A	-
Moisture	N	2030	%	0.020	10
рН	М	2010		4.0	9.5
Boron (Hot Water Soluble)	М	2120	mg/kg	0.40	0.78
Sulphate (2:1 Water Soluble) as SO4	М	2120	g/l	0.010	< 0.010
Cyanide (Total)	М	2300	mg/kg	0.50	< 0.50
Arsenic	М	2450	mg/kg	1.0	21
Cadmium	М	2450	mg/kg	0.10	0.47
Chromium	М	2450	mg/kg	1.0	13
Copper	М	2450	mg/kg	0.50	12
Mercury	М	2450	mg/kg	0.10	< 0.10
Nickel	М	2450	mg/kg	0.50	17
Lead	М	2450	mg/kg	0.50	42
Zinc	М	2450	mg/kg	0.50	34
Organic Matter	М	2625	%	0.40	1.3
Total TPH >C6-C40	М	2670	mg/kg	10	< 10
Naphthalene	М	2700	mg/kg	0.10	< 0.10
Acenaphthylene	М	2700	mg/kg	0.10	< 0.10
Acenaphthene	М	2700	mg/kg	0.10	< 0.10
Fluorene	М	2700	mg/kg	0.10	< 0.10
Phenanthrene	М	2700	0 0	0.10	< 0.10
Anthracene	М	2700	mg/kg	0.10	< 0.10
Fluoranthene	М	2700	mg/kg	0.10	< 0.10
Pyrene	М	2700	mg/kg	0.10	< 0.10
Benzo[a]anthracene	М	2700	mg/kg	0.10	< 0.10
Chrysene	М	2700	mg/kg	0.10	< 0.10
Benzo[b]fluoranthene	М	2700	mg/kg	0.10	< 0.10
Benzo[k]fluoranthene	М	2700	mg/kg	0.10	< 0.10
Benzo[a]pyrene	М	2700	mg/kg	0.10	< 0.10
Indeno(1,2,3-c,d)Pyrene	М	2700	mg/kg	0.10	< 0.10
Dibenz(a,h)Anthracene	М	2700	mg/kg	0.10	< 0.10
Benzo[g,h,i]perylene	М	2700	mg/kg	0.10	< 0.10
Coronene	N	2700	mg/kg	0.10	< 0.10
Total Of 17 PAH's	N	2700	mg/kg	2.0	< 2.0
Total Phenols	М	2920	mg/kg	0.30	< 0.30

Chemtest Job No:	20-28443				Landfill	Waste Acceptanc	e Criteria
Chemtest Sample ID: Sample Ref:	1084161					Limits Stable, Non-	
Sample ID: Sample Location:	R13-CP03					reactive	Hazardous
Top Depth(m):	1.0				Inert Waste	waste in non-	Waste
Bottom Depth(m):	1.0				Landfill	hazardous	Landfill
Sampling Date:	19-Oct-2020					Landfill	
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	М	%	0.75	3	5	6
Loss on Ignition							10
Total BTEX	2760	М	mg/kg	< 0.010	6		
Total PCBs (7 Congeners)	2815	М	mg/kg	< 0.10	1		
TPH Total WAC (Mineral Oil)	2670	М	mg/kg	< 10	500		
Total (Of 17) PAH's	2700	Ν	mg/kg	< 2.0	100		
pH						>6	
Acid Neutralisation Capacity						To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	10:1 Eluate	Limit values	for compliance	eaching test
			mg/l	mg/kg	using B	S EN 12457 at L/	S 10 I/kg
Arsenic	1450	U	0.0038	< 0.050	0.5	2	25
Barium	1450	U	0.0017	< 0.50	20	100	300
Cadmium	1450	U	< 0.00010	< 0.010	0.04	1	5
Chromium	1450	U	< 0.0010	< 0.050	0.5	10	70
Copper	1450	U	0.0018	< 0.050	2	50	100
Mercury	1450	U	< 0.00050	< 0.0050	0.01	0.2	2
Molybdenum	1450	U	< 0.0010	< 0.050	0.5	10	30
Nickel	1450	U	< 0.0010	< 0.050	0.4	10	40
Lead	1450	U	< 0.0010	< 0.010	0.5	10	50
Antimony	1450	U	< 0.0010	< 0.010	0.06	0.7	5
Selenium	1450	U	< 0.0010	< 0.010	0.1	0.5	7
Zinc	1450	U	< 0.0010	< 0.50	4	50	200
Chloride	1220	U	< 1.0	< 10	800	15000	25000
Fluoride	1220	U	0.14	1.4	10	150	500
Sulphate	1220	U	2.1	21	1000	20000	50000
Total Dissolved Solids	1020	Ν	57	570	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.30	1	-	-
Dissolved Organic Carbon	1610	U	4.9	< 50	500	800	1000

Solid Information	
Dry mass of test portion/kg	0.090
Moisture (%)	10

Waste Acceptance Criteria

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

Test Methods

SOP	Title	Parameters included	Method summary
1020	Electrical Conductivity and Total Dissolved Solids (TDS) in Waters	Electrical Conductivity and Total Dissolved Solids (TDS) in Waters	Conductivity Meter
1220	Anions, Alkalinity & Ammonium in Waters	Fluoride; Chloride; Nitrite; Nitrate; Total; Oxidisable Nitrogen (TON); Sulfate; Phosphate; Alkalinity; Ammonium	Automated colorimetric analysis using 'Aquakem 600' Discrete Analyser.
1450	Metals in Waters by ICP-MS	Metals, including: Antimony; Arsenic; Barium; Beryllium; Boron; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Tin; Vanadium; Zinc	Filtration of samples followed by direct determination by inductively coupled plasma mass spectrometry (ICP-MS).
1610	Total/Dissolved Organic Carbon in Waters	Organic Carbon	TOC Analyser using Catalytic Oxidation
1920	Phenols in Waters by HPLC	Phenolic compounds including: Phenol, Cresols, Xylenols, Trimethylphenols Note: Chlorophenols are excluded.	Determination by High Performance Liquid Chromatography (HPLC) using electrochemical detection.
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2040	Soil Description(Requirement of MCERTS)	Soil description	As received soil is described based upon BS5930
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2192	Asbestos	Asbestos	Polarised light microscopy / Gravimetry
2300	Cyanides & Thiocyanate in Soils	Free (or easy liberatable) Cyanide; total Cyanide; complex Cyanide; Thiocyanate	Allkaline extraction followed by colorimetric determination using Automated Flow Injection Analyser.
2450	Acid Soluble Metals in Soils	Metals, including: Arsenic; Barium; Beryllium; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Vanadium; Zinc	Acid digestion followed by determination of metals in extract by ICP-MS.
2625	Total Organic Carbon in Soils	Total organic Carbon (TOC)	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2670	Total Petroleum Hydrocarbons (TPH) in Soils by GC-FID	TPH (C6–C40); optional carbon banding, e.g. 3- band – GRO, DRO & LRO*TPH C8–C40	Dichloromethane extraction / GC-FID
2700	Speciated Polynuclear Aromatic Hydrocarbons (PAH) in Soil by GC-FID	Acenaphthene; Acenaphthylene; Anthracene; Benzo[a]Anthracene; Benzo[a]Pyrene; Benzo[b]Fluoranthene; Benzo[ghi]Perylene; Benzo[k]Fluoranthene; Chrysene; Dibenz[ah]Anthracene; Fluoranthene; Fluorene; Indeno[123cd]Pyrene; Naphthalene; Phenanthrene; Pyrene	Dichloromethane extraction / GC-FID (GC-FID detection is non-selective and can be subject to interference from co-eluting compounds)
2760	Volatile Organic Compounds (VOCs) in Soils by Headspace GC-MS	Volatile organic compounds, including BTEX and halogenated Aliphatic/Aromatics.(cf. USEPA Method 8260)*please refer to UKAS schedule	Automated headspace gas chromatographic (GC) analysis of a soil sample, as received, with mass spectrometric (MS) detection of volatile organic compounds.
2815	Polychlorinated Biphenyls (PCB) ICES7Congeners in Soils by GC-MS	ICES7 PCB congeners	Acetone/Hexane extraction / GC-MS
2920	Phenols in Soils by HPLC	Phenolic compounds including Resorcinol, Phenol, Methylphenols, Dimethylphenols, 1- Naphthol and TrimethylphenolsNote: chlorophenols are excluded.	60:40 methanol/water mixture extraction, followed by HPLC determination using electrochemical detection.
640	Characterisation of Waste (Leaching C10)	Waste material including soil, sludges and granular waste	ComplianceTest for Leaching of Granular Waste Material and Sludge

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S	This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
SN	This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
Т	This analysis has been subcontracted to an unaccredited laboratory
I/S	Insufficient Sample
U/S	Unsuitable Sample
N/E	not evaluated
<	"less than"
>	"greater than"
	Comments or interpretations are beyond the scope of UKAS accreditation The results relate only to the items tested
	Uncertainty of measurement for the determinands tested are available upon request
	None of the results in this report have been recovery corrected
	All results are expressed on a dry weight basis
	The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis All Asbestos testing is performed at the indicated laboratory Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

Sample Deviation Codes

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

Sample Retention and Disposal

All soil samples will be retained for a period of 45 days from the date of receipt All water samples will be retained for 14 days from the date of receipt Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>

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Chemtest

Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL Tel: 01638 606070 Email: info@chemtest.com

Report No.:	20-28569-1		
Initial Date of Issue:	27-Oct-2020		
Client	Causeway Geotech Ltd		
Client Address:	8 Drumahiskey Road Balnamore Ballymoney County Antrim BT53 7QL		
Contact(s):	Carin Cornwall Colm Hurley Darren O'Mahony Gabriella Horan Joe Gervin John Cameron Lucy Newland Martin Gardiner Matthew Gilbert Neil Haggan Paul Dunlop Sean Ross Stephen Franey Stephen McCracken Stephen Watson Stuart Abraham Thomas McAllis		
Project	20-0399E Bus Connects Route 13		
Quotation No.:	Q20-21063	Date Received:	22-Oct-2020
Order No.:		Date Instructed:	22-Oct-2020
No. of Samples:	1		
Turnaround (Wkdays):	5	Results Due:	28-Oct-2020
Date Approved:	27-Oct-2020		
Approved By:			
Ula Marine			

Details:

Glynn Harvey, Technical Manager



🔅 eurofins

Chemtest

Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL Tel: 01638 606070 Email: info@chemtest.com Project: 20-0399E Bus Connects Route 13

Client: Causeway Geotech Ltd		Che	ntest Jo	ob No.:	20-28569
Quotation No.: Q20-21063	0	Chemte	st Sam	ple ID.:	1084750
		Sa	ample Lo	ocation:	R13-SLT03
				e Type:	SOIL
	Top Depth (m):		0.40		
	Date Sampled:		20-Oct-2020		
				os Lab:	COVENTRY
Determinand	Accred.	SOP	Units	LOD	
АСМ Туре	U	2192		N/A	-
Asbestos Identification	U	2192		N/A	No Asbestos Detected
ACM Detection Stage	U	2192		N/A	-
Moisture	N	2030	%	0.020	11
рН	U	2010		4.0	8.9
Boron (Hot Water Soluble)	U	2120	mg/kg	0.40	0.43
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	< 0.010
Cyanide (Total)	U	2300	mg/kg	0.50	< 0.50
Arsenic	U	2450	mg/kg	1.0	15
Cadmium	U	2450	mg/kg	0.10	1.6
Chromium	U	2450	mg/kg	1.0	12
Copper	U	2450	mg/kg	0.50	24
Mercury	U	2450	mg/kg	0.10	< 0.10
Nickel	U	2450	mg/kg	0.50	35
Lead	U	2450	mg/kg	0.50	69
Zinc	U	2450	mg/kg	0.50	77
Organic Matter	U	2625	%	0.40	3.1
Total TPH >C6-C40	U	2670	0 0	10	< 10
Naphthalene	U	2700	mg/kg	0.10	< 0.10
Acenaphthylene	U	2700	5	0.10	< 0.10
Acenaphthene	U	2700	0	0.10	< 0.10
Fluorene	U	2700	mg/kg	0.10	< 0.10
Phenanthrene	U	2700	0 0	0.10	< 0.10
Anthracene	U	2700	mg/kg	0.10	< 0.10
Fluoranthene	U	2700		0.10	< 0.10
Pyrene	U	2700	mg/kg	0.10	< 0.10
Benzo[a]anthracene	U	2700		0.10	< 0.10
Chrysene	U	2700	0 0	0.10	< 0.10
Benzo[b]fluoranthene	U	2700	0 0	0.10	< 0.10
Benzo[k]fluoranthene	U	2700	0 0	0.10	< 0.10
Benzo[a]pyrene	U	2700	mg/kg	0.10	< 0.10
Indeno(1,2,3-c,d)Pyrene	U	2700		0.10	< 0.10
Dibenz(a,h)Anthracene	U	2700	0 0	0.10	< 0.10
Benzo[g,h,i]perylene	U	2700	0 0	0.10	< 0.10
Coronene	N	2700	0 0	0.10	< 0.10
Total Of 17 PAH's	N	2700	mg/kg	2.0	< 2.0
Total Phenols	U	2920	mg/kg	0.30	< 0.30

Test Methods

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APPENDIX G SPT HAMMER ENERGY MEASUREMENT REPORT



SPT Hammer Energy Test Report

in accordance with BSEN ISO 22476-3:2005

RH19 4QA	Test Operator:	
East Grinstead West Sussex	File Name:	.0643.spt
Stuart Way	Report Date:	03/03/2020
Keeble House	Test Date:	22/02/2020
Southern Testing	SPT Hammer Ref:	.0643

Instrumented Rod Data

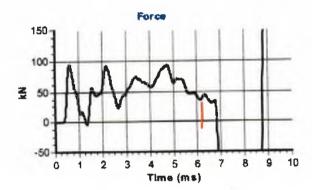
54
6.0
200
6458
9 607

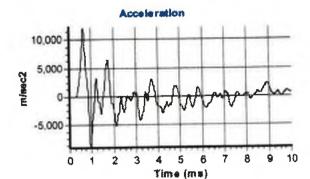
SPT Hammer Information

Hammer Mass m (kg):	63.5
Falling Height h (mm):	760
SPT String Length L (m):	10.0

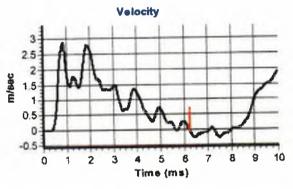
Comments / Location

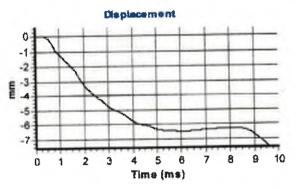
BALLEYMONEY











Calculations

Energy Ratio E , (%):	85
Measured Energy E _{meas} (J):	400
Theoretical Energy Etheor (3):	473
Area of Rod A (mm2):	905



Signed: **Neil Burrows** Title: Field Operations Manager

The recommended calibration interval is 12 months



SPT Hammer Energy Test Report

in accordance with BSEN ISO 22476-3:2005

	mant 41	is monthing	
RH19 4QA	Test Operator:	NPB	
East Grinstead West Sussex	File Name:	.T7.spt	
Stuart Way	Report Date:	03/03/2020	
Keeble House	Test Date:	22/02/2020	
Southern Testing	SPT Hammer Ref:	.17	

Instrumented Rod Data

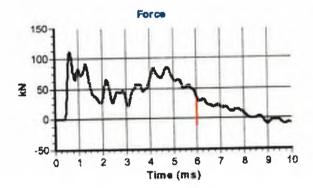
Diameter dr (mm):	54
Wall Thickness t, (mm):	6.0
Assumed Modulus E_a (GPa):	200
Accelerometer No.1:	6458
Accelerometer No.2:	9607

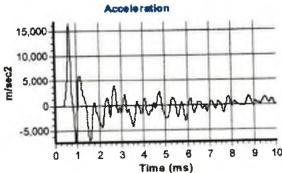
SPT Hammer Information

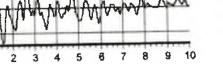
Hammer Mass m (kg):	63.5
Falling Height h (mm):	760
SPT String Length L (m):	10.0

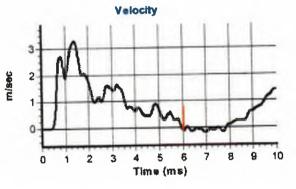
Comments / Location

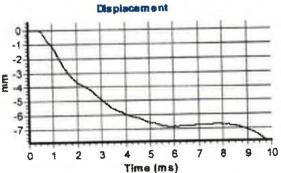
BALLEYMONEY











Calculations

905 Area of Rod A (mm2): Theoretical Energy Etheor (J): 473 399 Measured Energy E_{meas} (J): Energy Ratio E, (%): 84

Signed: Neil Burrows Field Operations Manager Title:

The recommended calibration interval is 12 months